

TRAFFIC CONTROL BY COMPUTER SIMULATION
AT
THE MINISTRY OF ECONOMY INTERSECTION

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SUBMITTED IN PARTIAL FULFILMENT OF THE
REQUIREMENTS FOR THE DEGREE OF MASTER
OF SCIENCE IN INDUSTRIAL ENGINEERING
FACULTY OF GRADUATE STUDIES
UNIVERSITY OF JORDAN

FEBRUARY 1991

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The examining committee consider this thesis satisfactory
and acceptable for the award of the degree of master of
science in Industrial Engineering.

Approved :



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Date : FEB. 26, 1991

ملخص

بعض التقاطعات المرورية في الأردن وخاصة في العاصمة عمان، تعاني من ازمتات خانقة وتأخر ملحوظ، وفترة انتظار طويلة نسبياً للعبور من خلال التقاطع، وكان من أهم هذه التقاطعات تقاطع وزارة الصناعة والتجارة.

حلول كثيرة مقترحة لحل مثل هذه المشاكل، ولكن اختيار حل من آخر يعتمد على مدى نجاعة مثل هذا الحل عن غيره. ولمعرفة أن نتائج مثل هذا الحل مقبولة، هنالك طرق علمية هندسية ومن أهم هذه الطرق وأنجعها وأسهلها هي المحاكاة بواسطة الكمبيوتر.

لقد استخدمت المحاكاة في هذه الرسالة لنرى أفضل حل من ثلاثة حلول مقترحة لهذا التقاطع. حيث كان الحل الأول تنظيم وجدولة وقت الاشارات الضوئية حيث تعطي أفضل توزيع للضوء الأخضر، والحل الثاني زيادة في عرض شارع العربي وبئر السبع، والحل الثالث إقامة نفق على شارع الملكة نور.

النتائج بينت أن الحل الأول يمكن أن يعمل به لغاية عام ١٩٩٣م حيث بعد هذا الوقت يصبح عدد السيارات كبيراً بحيث لا يمكن التعامل معها حسب الواقع الموجود. أما في الحل الثاني وبتطبيقه حتى عام ٢٠٠٠م وجد أنه غير فعال لتلك الفترة مع وجود بعض المشاكل في شراء أرض من أصحابها، أما الحل الثالث فكان مثالياً من حيث أن فترة الانتظار أصبحت غير مذكورة نسبياً وكذلك انتهاء الازمة أصبح واضحاً تماماً وذلك بتطبيقه لعام ٢٠٠٠، وذلك بغض النظر عن التكاليف المالية.

إن استخدام المحاكاة وهي إحدى الطرق العلمية الجديدة والحديثة، في معرفة النتائج المستقبلية المترتبة على أي تغيير أو تجديد دونما حصول ذلك التغيير أو التجديد، سوف يساعد أصحاب القرارات في اتخاذ قراراتهم على ضوء تلك النتائج.

ABSTRACT

Delay, and long queues are noticed in many intersections in Amman. Ministry of Economy intersection is a good example of such delays. Many alternate solutions were proposed to solve the problem of that intersection. In order to predict the consequences and the performance of such alternatives, many methods, tools, and techniques are available, SIMULATION is one of the simplest and practical methods used in such issues.

Simulation technique by GPSS, is used to simulate three alternative solutions; the first one is to assign the best cycle time for the traffic light signals. The second is to increase the number of lanes in branch 2 and 4 . And the third is to construct a tunnel (underpass) along the Queen Noor street.

The results show that, the first alternative can be applied up to year 1993, and after that time, the intersection can not handle the traffic flow. The second alternative can not be efficient after year 2000, and some difficulties in acquiring the land will be encountered, but the third alternative shows the best and efficient solution, without cost consideration

The use of simulation can be very helpful for the decision makers, because they can see the effects of any changes without actual implementation of these changes.

ACKNOWLEDGMENTS

I wish to express my heartfelt gratitude to professor Parakash Mahajan, the original supervisor of the thesis, for his prudent guidance, supervision, and suggestions, without which this work would not have been accomplished.

I am greatly indebted and grateful to professor Arun Walvekar who continued the supervision and guidance during professor Mahajan's unexpected absence.

I would sincerely like to express my great thanks to the members of examination committee, Dr. Alaa Shahab, and Dr. Bassam Talhouni, for their constructive and useful remarks and comments.

My genuine gratitude and appreciation to my family, specially, my parents for their support and encouragement. I would like to record my thanks to the staff of computer center in Faculty Of Engineering.

Special thanks are due to my colleagues, and to the staff in Industrial Engineering department. I am also grateful to the Greater Amman municipality for providing the needed data for this work.

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CHAPTER ONE

INTRODUCTION

In Amman , the capital of Jordan , there is a large number of vehicles compared with it's area ,and the traffic flow faces many problems .Some of these problems are the intersections and conjunctions . In many intersections there are very long queues of vehicles waiting to pass the traffic light signal .

Year by year ,the traffic flow increases and most of the roads and routes have not changed ; that causes delays and long waiting times at the intersections .These delays will bother and annoy the people who use such intersections , and this will be reflected on their performance of the work and the activities .

Many solutions are available for such problems , but which one to pick ? And why ? And how can you check that it will be the best one before implementing it ?

Traffic simulation , a tool used by traffic engineers in the analysis of roadway capital investment and traffic control management , provides valuable information to decision makers by predicting the likely effects of traffic patterns or geometric changes of a roadway before the changes actually occur .

A big mistake was done when Abd Al-Naser interchange was designed . It was that the designers didn't simulate the interchange before implementing it , in order to see how it will work and if there are any problems that would arise .

Simulation results may be used to decide whether to proceed with the change , modify it , or abandon it . Simulation may determine the most effective way to spend available funds .[11] .

This is the big advantage of simulation : to predict the results of the project without executing it .

1.1 PROBLEM DEFINITION

The intersection which was selected to study is beside the Ministry of Economy and called the " Ministry of Economy Intersection " .

This intersection is selected to be studied, because of the following reasons:

- 1- It lies in a very important part in Amman i.e. center of Amman (among government departments,banks,markets ,..etc.) .
- 2- It has a very large flow of traffic .
- 3- It faces a real problem of long queues and delay times

4- It connects many major parts in Amman within a small travel distance .

By observing that intersection , specially in the morning and afternoon periods , one can easily notice that there is a problem and it must be solved .

This intersection consists of four branches and as follows :

- 1 - Queen Noor street southern.
- 2 - Beir Sab'a street .
- 3 - Al Arabi street .
- 4 - Queen Noor street northern .

The sketch of the intersection is shown in figure (1) . A very big problem in this intersection is the queue length in Beir Sab'a street and Al-Arabi street .That is because each of these two streets has only two lanes and the period of the green light time to pass is also small,while in Queen Noor street there are four lanes which can help in minimizing the delay and waiting times in the queue .

There are many alternate solutions , such as : changing the geometry of the intersection by increasing number of lanes ,building bridges , constructing a tunnel , or scheduling the existing traffic signals for reasonable queue length and delay times .

Of course any changing in intersection geometry will cost

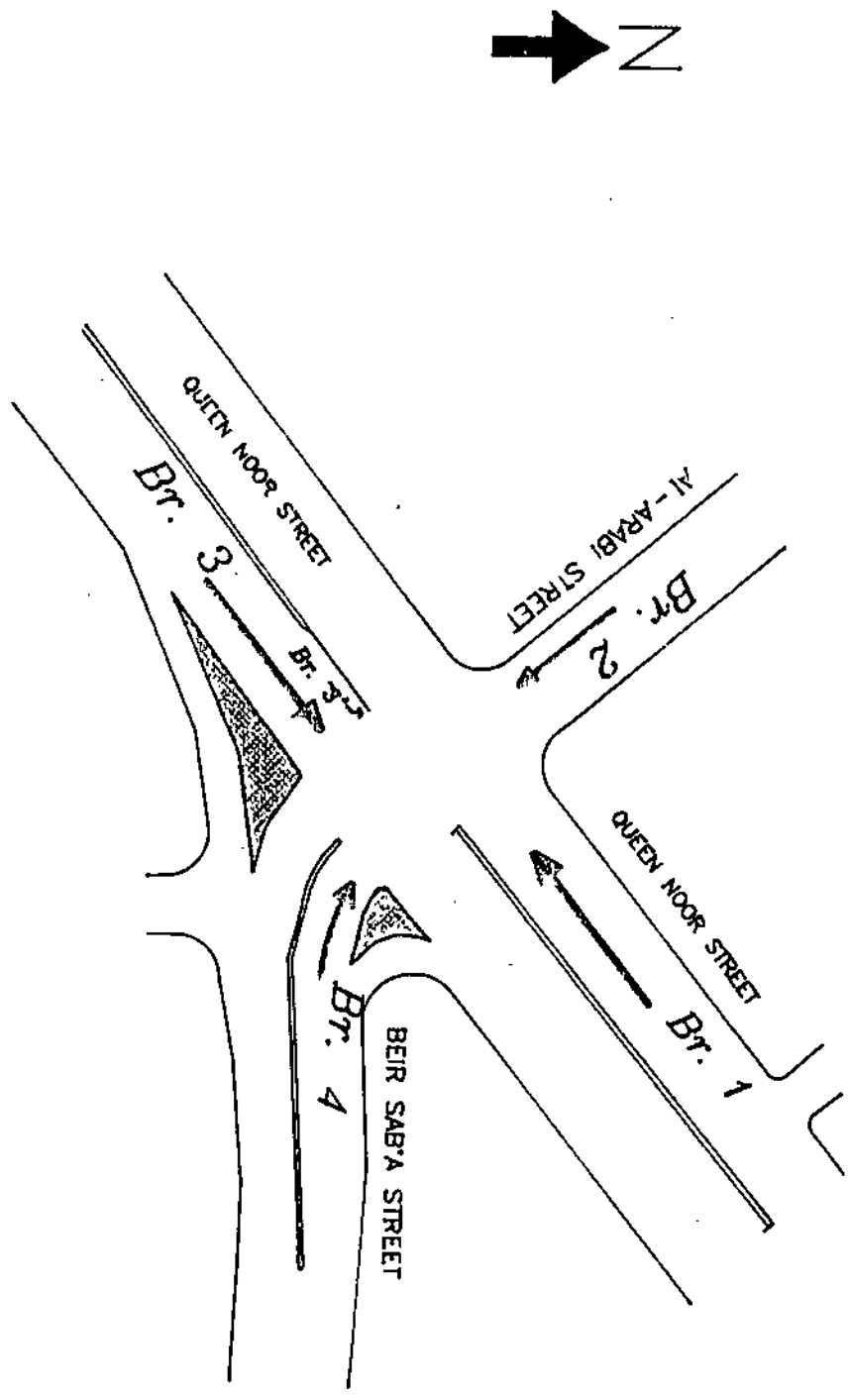


FIG. 1.1 THE MINISTRY OF ECONOMY INTERSECTION.

a lot of money and will make some troubles with land owners beside the intersection .

So in this thesis we will try to solve this problem by simulating each alternative, and comparing the results for each of them, then picking the best one.

the results of each alternate solution and picking the best one of them .

1.2 OBJECTIVES

The objectives of this research are :

- Study the existing situation of the intersection
- Schedule and regulate the traffic signals to reduce the queue lengths and delay times to get easy and smooth flow of the traffic .
- Simulate the alternate solutions and analyze the results .The results of the study will be compared for the performance of each alternative, based on the queue length and delay time in each branch.

CHAPTER TWO

LITERATURE SURVEY

Many researches were done on the subject of traffic control by computer simulation, and many simulation models were developed.

University of Bradford developed some computer simulation models for the simulation of traffic flow at highway junctions controlled by traffic signals and have been used to estimate cycle times which produce minimum delay .[7].

The required input to the program is : simulation time , average and minimum headway of arriving vehicles , demand and saturation flow and cycle and lost times . the output of the program is : average delays to vehicles on each approach , queue on each approach , and total intersection delay . The output and results were validated against the Webster delay formula and Catling queue length formula .[6].

Another simulation model program called (INTERCON) ,(by Roger Plum and Panos Michalopoulos)was developed .[8]. The type of control used in this model is the pretimed signal control with a fixed cycle length , phasing sequence and phase times .

Vehicles arrivals are assumed to occur randomly with each

lane . The time used as the arrival time for each vehicle is the time at which the vehicle arrives at the back of the queue , or if no queue is present at the stop line .

For approach that have more than one lane accommodation through traffic , the arrivals of through vehicles are distributed between the lanes so that the total demand in each lane is as balanced as possible with the demand on other lanes of the approach .

Vehicles departures are function of several parameters entered by the user . The actual departure rate is a function of :

- saturation flow .
- lost time . It was assumed that the first five vehicles in a platoon can be affected by a lost time due to driver reaction and acceleration time .
- Average gap in the opposing flow that the driver makes a left turn feels is adequate .

2.1 PRETIMED SIGNALS

This common type of traffic control signal assigns the right of way at an intersection according to predetermined schedule . The time interval for each signal indication in

the cycle time is of fixed length .[3].

2.2 SIGNAL TIMING

Timing calculation are based on traffic requirements. Cycle length during off-peak-periods should be as short as possible (40-100)sec..

Larger cycles are used during peak periods to provide more green time for the major street , to permit larger platoons in the peak direction and /or to reduce the number of starting delays .

General procedure is presented to calculate the signal timing [3]:

- Yellow change intervals , based on approach speeds

 - 3 sec. is used for speeds up to 55 km/h .

 - 4 sec. is used for speeds (55-80) km/h .

 - 5 sec, is used for speeds more than 80 km/h .

- Additional clearance time : At wide intersection or approach speed is very high , some drivers may be in a dilemma zone , where they can neither stop safely nor clear the intersection . To determine if an all red clearance

interval is necessary to avoid this situation , the following equation can be used:

$$Y = T + \frac{V}{2A} + \frac{W+L}{V} \dots\dots\dots 2.1$$

Where

- Y: non dilemma clearance interval (s) .
- T: perception-reaction time normally 1 sec. .
- V: approach speed (m/sec.) .
- A: deceleration rate , normally 3 m/sec . .
- W: width of the intersection (m) .
- L: length of the vehicle , normally 6 m .

If Y is greater than the value for the yellow change interval , then the difference is the required ALL-RED interval .

- Pedestrian clearance time : pedestrian walking speed ranges 4-3.5 ft./sec. .
- Minimum green time (MGT): it is equal to pedestrian clearance time - yellow change interval + initial interval time (INT) . Minimum green time must be

greater than 15 sec..

- Initial interval time (INT):

1. with pedestrian signals INT=the walk period must be > 7 sec. .

2. without pedestrian signals , INT not <5 sec. .

- Green time (GT) : based on these minimum , in proportion to the approach volume in critical lane , on each street during the heaviest hour . GT > MGT .

- Cycle time is to be adjusted to the next higher 5 sec. interval and redistribute extra green time .

An other method to calculate the theoretical cycle length for random - arrival headway is [5] :

$$C = \frac{3600 * \sum_{x=1}^n K_x}{3600 - \sum_{x=1}^n V_x DK_x} \dots\dots\dots 2.2$$

where

C : cycle length (s)

K : 4.75 sec. for typical passenger car .

Dk: constant value = 2.1 sec.

V : volume in vehicles per hour .

n : number of phases .

The above equation gives us theoretical value for the cycle length . Studies have shown that there are two phenomena occurring at nearly each intersection :

1- left turn

2- presence of trucks and other large vehicles

in the traffic flow .

Studies have shown that because of delay enforced by opposing traffic , left turn generally require an average of 1.3 additional sec to clear the intersection . Thus time for left turning vehicle $\equiv [(2.1+1.3)=1.6$ (time for straight through vehicle in sec.)][5] .

For the trucks or large vehicles, it has been found that each truck or bus consumes approximately 1.5 times the amount of departure time required for a passenger vehicle.

2.3 DELAY TIMES

When a red signal interrupts a traffic flow , the vehicles will stopped , and will require time to get started again , so additional number of vehicles may be stopped because of the starting performance of the queue which has been accumulated on the red signal, which will cause the delay [5].

The number of vehicles which will stopped or delayed and the duration of the delay are dependent on :

- red interval
- arrival headway in the flow , and
- the starting performance of the queue .

Let R = interval of stop signal ,sec.

n = number of stopped vehicle in R

i = any selected vehicle of the n vehicles

A = average headway of vehicles on arrival, sec.

D = headway of departure at intersection entry .

d_i = delay for vehicle i ,sec.

T = total delay sec. .

Note that D is variable for the first six vehicles and it will be constant for rest . So the delay for any vehicle becomes :

$$d_i = R - \frac{A(2i-1)}{2} + \sum_{x=1}^i D_x \dots\dots\dots 2.3$$

and the sum for the individual delays becomes :

$$T = nR - \frac{nA}{2} + \sum_{x=1}^n \sum_{i=x}^n D_x \dots\dots\dots 2.4$$

and it can be rewritten as :

$$T = nR - \frac{nA}{2} + \frac{2.1 n(n+1)}{2} + 3.7n - Q \dots\dots\dots 2.5$$

in which Q is always =3.5 for n >= 4

Some of the needed data is available , and the rest can be obtained by :

1-Calculating it from the equations described above .Or by,

2-Measuring it On the site .

We will use both approaches and compare between them .
This will be helpful to start with reasonable values .
For example the cycle time can be calculated from the previous equations or can be measured from the site .

CHAPTER THREE

RESEARCH PLAN

A simulation model of the traffic intersection will be developed to describe the traffic system for each alternate solution . In general ,each model represents queues , of the arrival process ,the service process , and the queue discipline [2] .

1 - The arrival process consists of vehicles , arrival rate , and the distribution of the arrivals . In this system the process is considered as a discrete process .

2 - The service process is characterized by distribution function of the time to serve the arrivals , and the number of arrivals . The server in this system is the traffic signals .

3 - The queue discipline describes the order in which the arrivals are served . In the model which will be developed , we will use " FIRST IN FIRST OUT " (FIFO) , and also include the characteristics of the system, maximum queue length.

3.1 ALTERNATE SOLUTIONS

We will consider the following alternatives :

- 1 - No changing in geometric configuration and schedule the traffic signals to keep the queue length and the delay times as short as possible
- 2 - Increase the number of lanes in Beir Sab'a and Al-Arabi streets from two lanes up to three or four lanes . This can be done by expropriating some land beside the intersection .
- 3 - Changing the geometry of the intersection to become a two level grade intersection . This can be done by building a bridge or constructing a tunnel in the Queen Noor street .

For each alternative a new model will be developed in order to fit the alternate solution and represent the real situation.

There are a lot of models developed to describe a queue systemssuchas :

$M/M/1/\alpha/\alpha$, $G/G/1/\alpha/\alpha$, $M/G/1$, $D/D/1$, ...etc. . For example the first model indicates : a single - server system that has an infinite population of potential arrivals , the inter-arrival times are and service time are exponentially distributed

3.2 COLLECTING DATA

As we know , data collection is one of the largest tasks in solving a real life problem, because it must be accurate, precise , and representing almost the existing situation.

The data will consist of geometric configuration , traffic count for each direction , cycle time of the traffic signals maximum queue length , and vehicle clearance time .

All the traffic counts are available in Amman Municipality these counts have been taken at 15 minute periods for the whole day . Other needed data will be collected from the site such as : width of the branch , number of lane in each branch , number of vehicles arrive at each branch and time for certain number of vehicles to pass through or turn at the intersection .

The counts of traffic flow which is available now is representing the flow in 1988 , but the flow will increase according to

- Normal traffic growth.
- Generated traffic .
- Development traffic .

In order to achieve good solution for long period of time these traffic flows will be projected and forecasted for

the next twenty years .

Specific values cannot be cited for traffic projection factors for use in design where no analysis is made of the separate factors of traffic growth .

Projection factors that would apply to a majority of highways improvements today for a 20 years period will be approximately doubled .[10] .

The data will be arranged , checked , and analyzed before applying to the computer simulation program . Then we will apply the data to the program and check if the program is performing properly.If it does,we proceed with the analysis ;if not we check our model and data again .

3.3 MODEL SOLUTION

A computer simulation program will be developed to solve the model using " General Purpose Simulation System " (GPSS) language

VAX terminals will be used to run the GPSSH software , because it provides large number of transactions , facilities and queues , which cannot be found on personal computers .

In the models we developed some variable parameters can be changed such as the cycle time and the phasing . The selection and arrangement of simultaneous flows of

movements is known as "phasing" . The objective of phasing is to accommodate all traffic movements with minimum delays .

Generally speaking , the number of distinct phases employed should be kept as minimum as possible . The selection of flows in each phase should develop the minimum frequency and severity of conflict , and the sequence of phases should minimize waste of time . Illustrative examples of phasing are shown below : [5] .

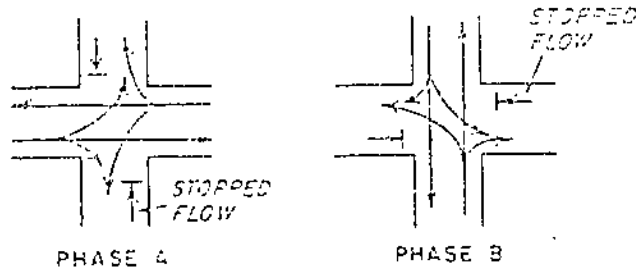
3.4 VALIDATION

Validation is the determination that the model is an acceptable in representing the real system , this can be achieved by comparing the results from the computer program with the existing situation at the intersection for the same data .

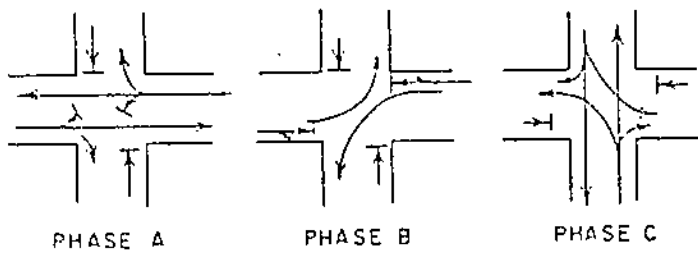
After checking and verifying the data , each alternate solution will be solved by the computer simulation program. Several runs for each alternative will be made , in each run we change some variable parameters and analyze the results until we get reasonable results .

Then the alternatives will be compared with each other to know which one is the most effective and cheapest .

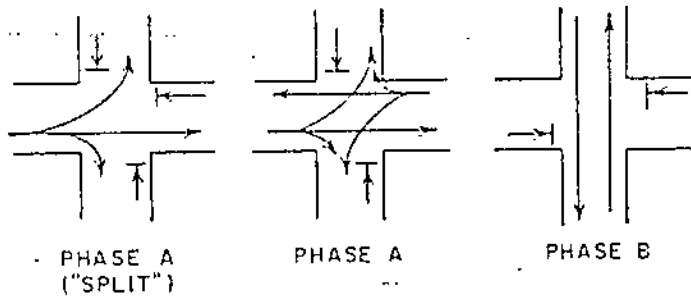
TRAFFIC ENGINEERING



USUAL PHASING OF TRAFFIC FLOW AT ORDINARY "RIGHT-ANGLE" INTERSECTION. TWO, TWO-WAY TRAFFIC STREAMS



USE OF THREE PHASES AT ORDINARY, "RIGHT-ANGLE" INTERSECTION. TWO, TWO-WAY TRAFFIC STREAMS WITH SEPARATE PHASE PROVIDED FOR LEFT TURNS



PHASE A ("SPLIT")
(TURNS IN ONE DIRECTION RECEIVE PRIORITY GREEN)

"TWO" PHASES, UTILIZING "SPLIT" FOR HEAVIER TURNING MOVEMENT

Phasing of traffic flows.

CHAPTER FOUR

DATA COLLECTION

4.1 INTRODUCTION

As it is known, data collection is one of the largest tasks in solving a real problem. The collected data must be accurate, precise, and representing the exactly the real existing situation. The collection and analysis of data is a critical step in the development of a simulation model. Any small error in data collection or analysis will lead to wrong results and invalidate the model.

4.2 REQUIRED DATA:

The data needed will consist of: geometric configuration, traffic flow in each direction (arrival process), percentage of the flow that pass-throughs, turn left, or turn right, cycle time of the traffic signals, vehicle clearance time (service time), No. of lanes in each direction, signal phasing, and width of each branch.

All the traffic data and counts had been taken from the Municipality of Greater Amman, these counts have been taken at 15-minute periods for the whole day. It had been noticed that there are two peaks, one in the morning period from 7:15-8:15, and the second is in the afternoon period from

CHAPTER FOUR

DATA COLLECTION

4.1 INTRODUCTION

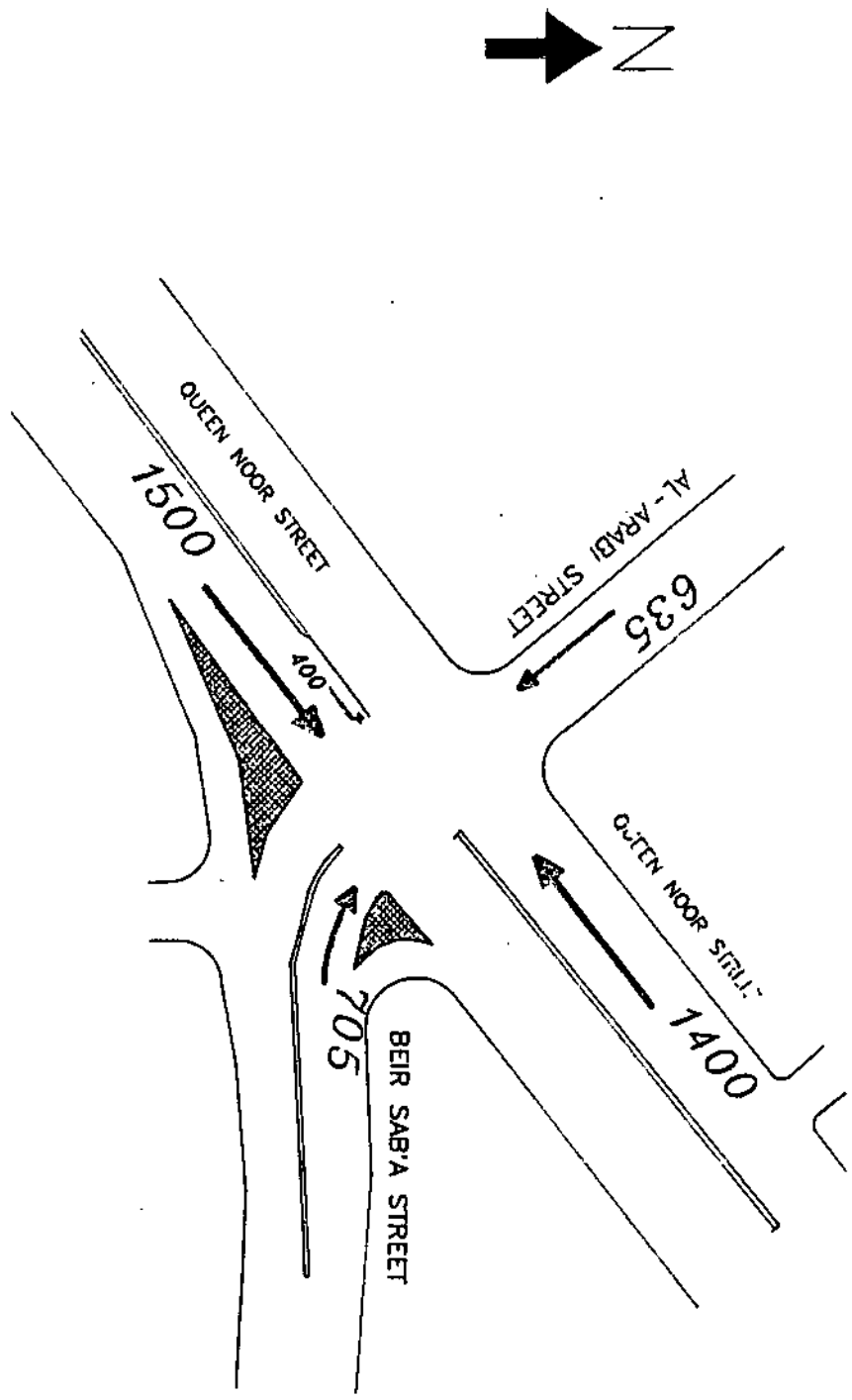
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FIG. 4.1 TRAFFIC FLOW IN PEAK HOUR IN YEAR 1988.



13:15-14:15. Figure 4.1 shows the flow in peak hour in 1988.

4.3 DATA COLLECTED ON SITE :

Not all the required data was available, the following were collected on site:

1- The existing signal phase: As shown in figure 4.3, branch number 2 starts to move while the other branches are waiting. Then branch number 3 and 3' start to move while the others are waiting. Branch number 3' stopped, then branch number 1 starts, while branch number 3 is still working. Then branch number 4 start to move while other branches are waiting, and so on .

For existing situation, the existing phase is the best one, because in any other phase, the traffic in branches 2 and 4 will be divided into two directions, the first one is pass-throughs, and the second is the left turn, and there is no enough space to do that in these two branches, because the width of them is very small, and that will cause disturbing to the drivers, and long queues in these two branches.

2- Cycle time for the existing signal phase: About twenty readings were taken, at different times and the average value of the cycle time=115 seconds.

3- Green time for each branch: Many readings were taken for

each branch and at different times, and the averages of the green times (G.T.) are listed below:

Green time for branch number 1 = 25 seconds.

Green time for branch number 2 = 20 seconds.

Green time for branch number 3 = 60 seconds.

Green time for branch number 3' = 30 seconds.

Green time for branch number 4 = 20 seconds.

These values and the cycle time are shown in figure 4.4.

4- Vehicle clearance time (service time): This means that how much time needed to cross the intersection, and this can be easily calculated by dividing the number of cars passing the intersection by the period of time taken for that number of cars to cross the intersection. But using this way the results will be approximated, not very accurate, and it will be averaged for the whole cars that cross the intersection without distinguished between the order of the car. A better way for calculating the service time is as follow:

a- Find the time needed for car number 1 in the queue to cross the intersection. take ten readings and find the average of them.

b- For car number 2 in the queue, do the same thing as in step a. And the same thing for car number 3,4,5,...and 10.

c- The service time = time needed for car number (i+1)-time

FIG. 4.2 TRAFFIC FLOW IN PEAK HOUR IN YEAR 1990.

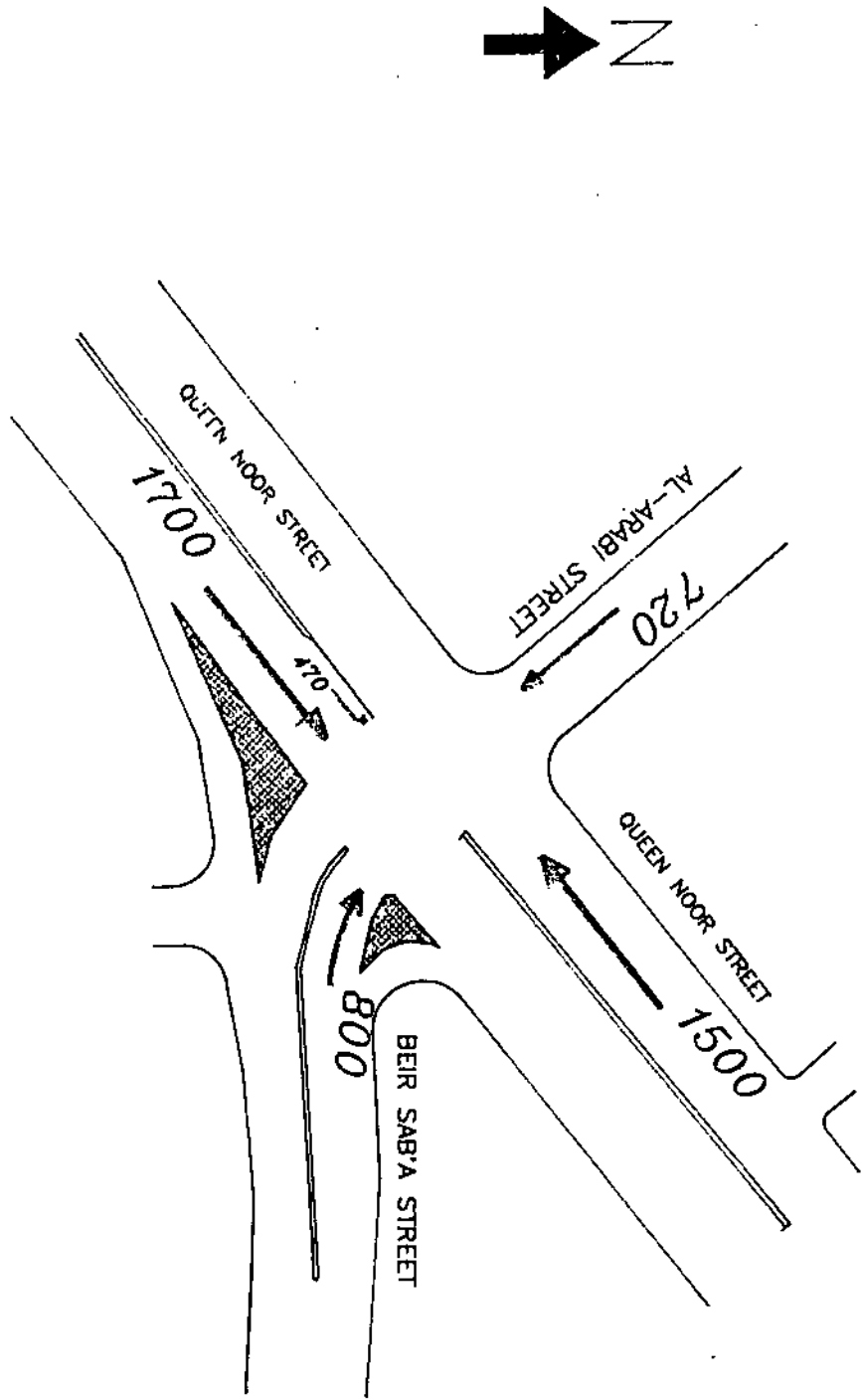
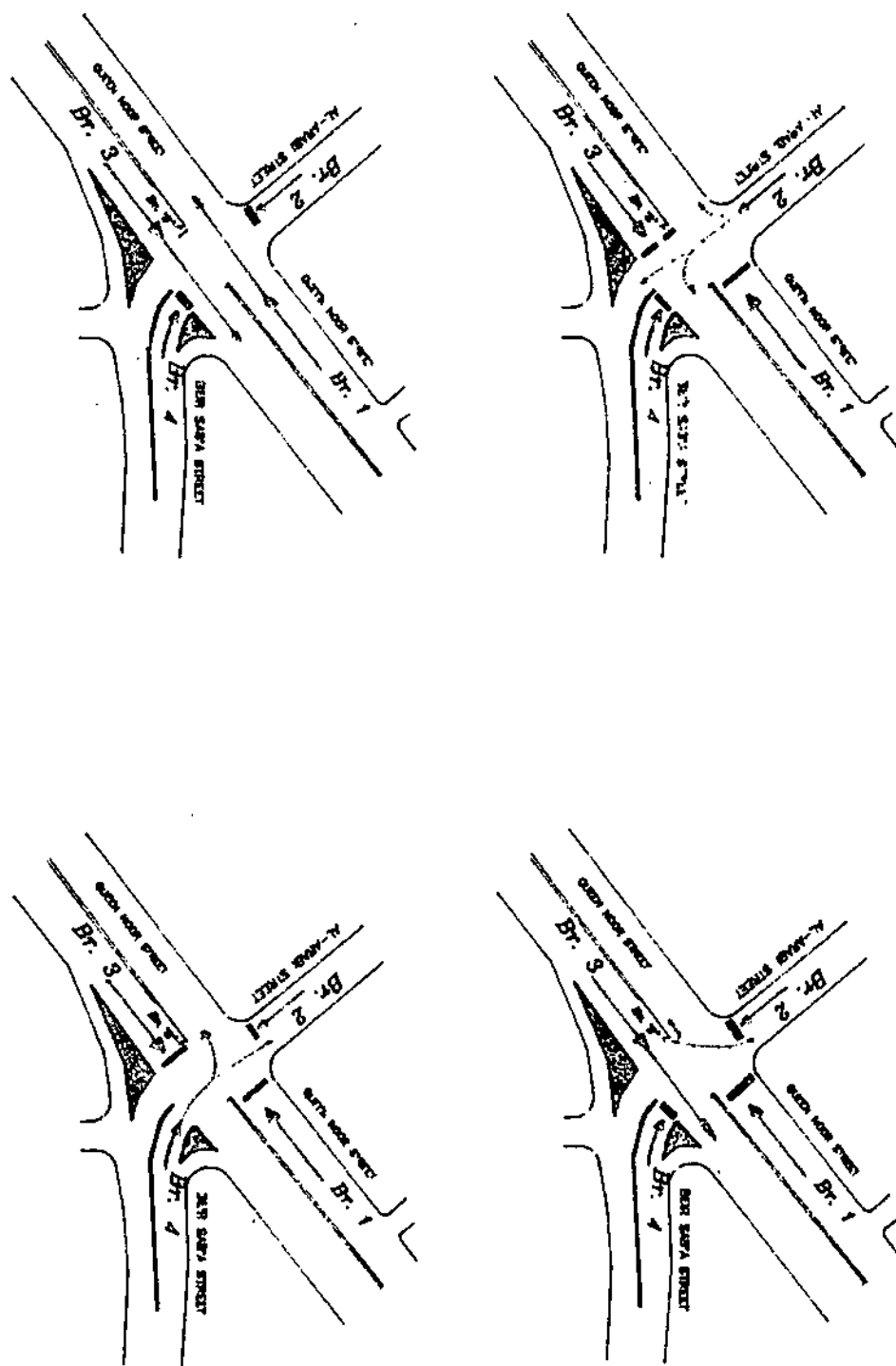


FIG. 4.3 EXISTING SIGNAL PHASE AT THE INTERSECTION.



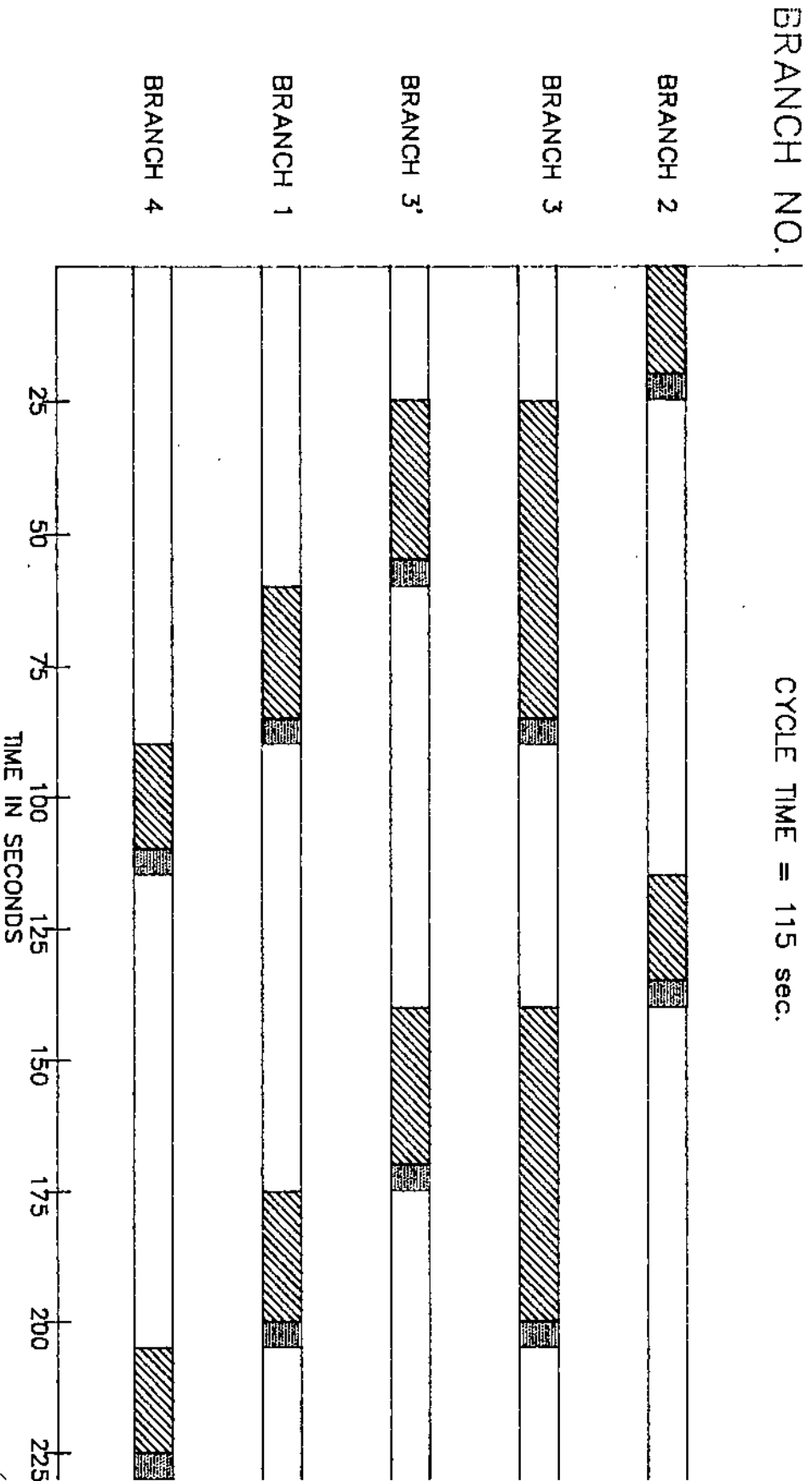


FIGURE 4.4 BAR CHART FOR THE GREEN TIME FOR EACH BRANCH.

TABLE 4.1 COLLECTED DATA FOR SERVICE TIME.

CAR NO.	PASS THROUGH		LEFT TURN		U-TURN	
	TIME NEEDED IN sec.	AV.	TIME NEEDED IN sec.	AV.	TIME NEEDED IN sec.	AV.
1	3,4,4	4.1	5,4,5	4.8	6,5,6	5.8
	4,5,4		4,5,5		6,5,5,5	
2	7,5,7,	6.8	8,8,7,	7.6	8,5,8,8,	9.1
	7,6,5		8,5,7,5		9,9,9,5	
3	8,9,9	8.6	10,10,10	10.4	12,12,12	12.3
	9,8,9		11,10,11		12,5,13	
4	11,11,	10.8	13,12,13	12.6	15,14,15	14.9
	10,11,		12,5,13		14,5,15	
5	12,13,	12.5	15,16,16	15.2	17,16,16	17.3
	13,12,		15,15,15		17,16,5	
6	14,13,	14.1	18,16,16	17.0	21,20,	20.2
	14,14		17,5,17		21,20,	
7	15,15	15.5	18,19,19	18.7	23,22,	22.8
	15,16		18,17,19		24,5,22	
8	16,17,	17.0	20,5,21	20.5	26,25	25.7
	17,18		20,21,5		24,5,25	
9	18,18	18.3	23,22,5	22.1	26,5,25	27.28
	18,19		21,5,21		29,28	
						27.5,28
						28.1

needed for car number(i).

Note that the first four or five cars will need larger service times, so skip these readings and find the average of the remaining readings for each car number. Tables 4.1 and 4.2 show these data. The data for the Right Turn was difficult to be taken and recorded, because there is no special lane for that movement, but it is approximately = 3 second.

TABLE 4.2 SUMMERY RESULTS FOR THE DATA IN TABLE 4.1.

	TIME NEEDED TO CROSS THE INTERSECTION FOR CAR No.									
	1	2	3	4	5	6	7	8	9	AV.
PASS THROUGH	4.1	2.7	1.8	2.2	1.7	1.6	1.4	1.5	1.3	1.45
LEFT TURN	4.8	2.8	2.8	2.3	2.6	1.8	1.7	1.8	1.6	1.75
U TURN	5.8	3.3	3.2	2.5	2.5	2.9	2.6	2.9	2.4	2.75

Cycle time and Green time for each branch can be calculated by any mentioned methods in chapter two.

4.4 DATA ANALYSIS :

All the counts and data took from the Municipality of Greater Amman was studied well, and tabulated in tables and figures. And also the other data that took from the site was tabulated.

4.4.1 FORECASTING:

All the traffic counts took from the Municipality of Greater Amman was representing the traffic flow counts in 1988, and when it will be applied to this time, it must be forecasted to this time (1990). When the problem of this intersection is to be solved, it must be solved not only for present time, but also for next certain period of the future time. In this thesis this period will be considered as ten years, so the data must be forecasted for year 2000.

Because there is no historical data available for the traffic flow counts for this intersection, the number of vehicles in Jordan will be good indicator to the traffic flow counts at this intersection. The historical data for the number of vehicles in Jordan is available in the Department of Traffic and Licences. These data was taken and tabulated as shown in table 4.3, and this data can be forecasted for any period of time and then can be used to be good indicator for the traffic flow in that period of time.

Linear regression method ,is selected to forecast the number of cars in JORDAN, by using the growth forecasting software called FOR.BAS.[12].The results of this forecasting are shown in the next page.

The forecasting linear model is representing in the following equation :

$$Y(T) = 27.18 + 17.85 T \dots\dots\dots 4.1$$

and by linear interpolation the flow counts can be calculated for the year 2000, the forecasted flow counts are shown in figure 4.6.

--- Regression Analysis ---

Name of the data set : th

Number of data values input : 12

Forecasting model :

$$Y(T) = 27.18182 + 17.25664 * T$$

Variance of forecast error : 38.53115

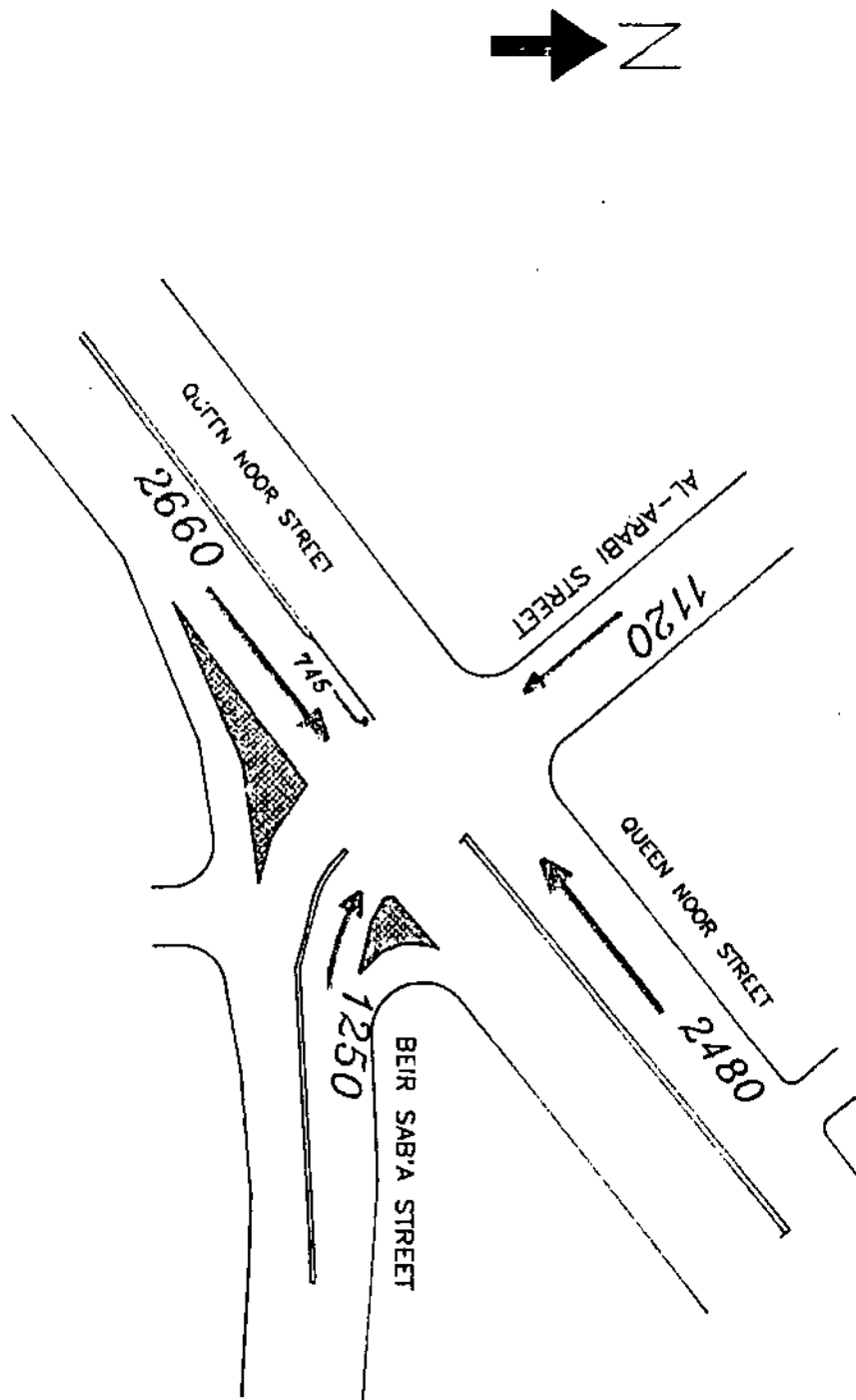
Mean absolute deviation : 5.035548

Average absolute percent error : 3.80773

TIME PERIOD	ACTUAL DATA	ESTIMATED VALUE	RESIDUAL
1	48.00	45.04	2.96
2	60.00	62.90	-2.90
3	77.00	80.75	-3.75
4	99.00	98.61	0.39
5	121.00	116.47	4.53
6	127.00	124.32	-7.32
7	147.00	152.18	-5.18
8	177.00	170.03	6.97
9	199.00	187.89	11.11
10	210.00	205.75	4.25
11	219.00	223.60	-4.60
12	235.00	241.46	-6.46

TIME PERIOD	FORECAST VALUE	95% CONTROL LIMITS	
		UPPER	LOWER
13	259.32	271.48	247.15
14	277.17	289.34	265.01
15	295.03	307.20	282.86
16	312.89	325.05	300.72
17	330.74	342.91	318.58
18	348.60	360.77	336.43
19	366.46	378.62	354.29
20	384.31	396.48	372.15
21	402.17	414.34	390.00
22	420.03	432.19	407.86
23	437.88	450.05	425.72
24	455.74	467.91	443.57
25	473.60	485.76	461.43

FIG. 4.5 FORECASTED TRAFFIC FLOW IN PEAK HOUR IN YEAR 2000.



CHAPTER FIVE

MODEL DEVELOPMENT

5.1 INTRODUCTION

A simulation model will be developed to represent the real system. In the model development, the description of the system will be made explicitly by quantifying the relationships among all the variables and the performance measures. In order to develop an accurate computer program which implements the model, the system and all of its elements must be fully understood.

5.2 DESCRIPTION OF THE SYSTEM

The system that will be studied, represents an isolated intersection, this isolated intersection consists of four branches, and because there are arrivals, services, and queues this system is considered as a queuing system.

Any queuing system consists of three elements:

- 1- An arrival process.
- 2- A service process.
- 3- A queuing discipline.[2].

In a queuing system the arrival process is characterized by the distribution of the time between the arrival of

successive customers. And in the system we are dealing with, the time between arrivals is exponentially distributed.

An exponential distribution plays a central role in queuing models, because it is a memoryless distribution. It is known that if the time between arrivals is exponentially distributed, with mean = θ , then the distribution of the number of arrivals during the unit time interval is a poisson distribution with mean = $1/\theta$.

The service process is characterized by the distribution function of the time to serve the arrivals, and the number of arrivals. In the system we are dealing with, the service process is considered as a uniform or constant function for each type of movement.

The queue discipline describes the order and the way in which arrivals are served. FIFO (First In First Out) and priority queues will be used in the system we are dealing with.

5.3 GENERAL PURPOSE SIMULATION SYSTEM (G P S S)

The General Purpose Simulation System (GPSS) language, will be used as a simulation language to build the computer model that representing the system we are dealing with.

GPSS is a highly structured, special-purpose simulation

language using the process-interaction approach and oriented toward queuing simulation.

In this language there are many concepts, as the block; which can be represented by pictorial symbol or by single statement. There are over than 45 standard blocks in GPSS, each block represents a specific action. A fundamental set of GPSS blocks is: GENERATE, TERMINATE, SEIZE, RELEASE, ADVANCE, QUEUE, and DEPART. In addition the following control statements are needed to execute even the simplest model: SIMULATE, START, and END. Each block in GPSS has a unique flow chart symbol. When the flow of transactions in the model is described using these symbols, converting the flow chart to computer code is accomplished by simply recording the corresponding statement for each symbol. [2].

An other concept, is the transaction: which represents active, dynamic entities may be pictured as flowing through the block diagram, and this will execute a GPSS model.

A third concept, is the simulation clock; the GPSS processor automatically maintains a simulation clock and it will advance the clock after finishing certain event or series of events. Note that GPSS automatically updates the simulation clock as required by the logic described in the model

5.4 APPROACH TAKEN IN BUILDING THE MODEL

When simulation begins, a transactions - customers are brought into the model, first; the branch number 1 will take the light as a green light (occupy the facility of crossing the intersection), and will be active to allow the vehicles to cross the intersection, while the other branches will be closed (in the red zone). Then after certain time the green light will be taken by the second branch, to allow the vehicles which were waiting in it's queues, to cross the intersection while the other branches are waiting (they are in red zone) in their queues. Then the third branch takes place in the green light zone. Then the fourth branch takes place in the green light zone. And the cycle will repeat it self again and so on.

This technique can be illustrated by using different priority levels for certain transactions. For example, when the first branch has occupy the green light zone for certain time, and this time is finished, so in order to let the first branch to leave the green light, a pseudo transaction with high priority will occupy the first branch until the cycle is finished and come back to the first branch. This technique is used for all the branches.

In branch number 1 there are four lanes, and four facilities are assigned to each lane. When a transaction is brought into this branch, first it will check the queue length for each lane and occupy the least of them. So before entering any queue in any branch, there will be a test to check and assign the smallest queue length, then entering that queue. Then spending certain time in the queue, then served by the facility. As mentioned before the service time is constant for each type of movement. Then the transaction will leave the system.

Now consider branch number 3. In this branch the flow split into two movements, the first one is passing through, while the second turns left, this second movement is considered as branch number 3'. The branch 3' also split into two movements, the first is turning left, and the second is turning back (U-tern). In this branch the transfer block is used to allow 12% of it's flow to turn back, and to give each type of movements it's exact service time.

In this model, in order to cancel the effect of randomness in several outputs, similar random number generators, and similar exponential function for each GENERATE block, were used. Also same seeds for each random number generator are assigned, by using the R-MULT block.

Also in order to reach a steady state situation, in each case the program was run seven times, and 60 minute for each time.

5.5 DEFINITIONS FOR THE ELEMENTS OF THE MODEL

TRANSACTIONS

MODEL SEGMENT 1	customers in branch 1
Model segment 1-1	red light for lane 1
Model segment 1-2	red light for lane 2
Model segment 1-3	red light for lane 3
Model segment 1-4	red light for lane 4
MODEL SEGMENT 2	customers in branch 2
Model segment 2-1	red light for lane 1
Model segment 2-2	red light for lane 2
MODEL SEGMENT 3	customers in branch 3
Model segment 3-1	red light for lane 1
Model segment 3-2	red light for lane 2
Model segment 3-3	red light for lane 3
MODEL SEGMENT 3'	customers in branch 3'
Model segment 3-1	red light for lane 1
MODEL SEGMENT 4	customers in branch 4
Model segment 4-1	red light for lane 1
Model segment 4-2	red light for lane 2
MODEL SEGMENT 5	timer.

FACILITIES 1-12	each lane for one facility
FUNCTIONS	
XPDS1	functions of exponential
XPDS2	distribution to describe
XPDS3	the arrival process for
XPDS4	customer arrivals in each
XPDS5	model segment..
QUEUES 1-12	queues used to gather
	statistics for each
	waiting lines a head
	of each facility.
TABLES 1-5	tables used to find % of
	time that queue content
	occupy the queues.

5.6 THE PROGRAM OUTPUT

The output of the program will include the following:

- 1- The utilization of each facility in the system. As mentioned earlier the facility will represent the lane. And also the total number of transactions interring each facility is represented.
- 2- The maximum queue content for each queue, average queue content, average time spent in the queue, the total entries

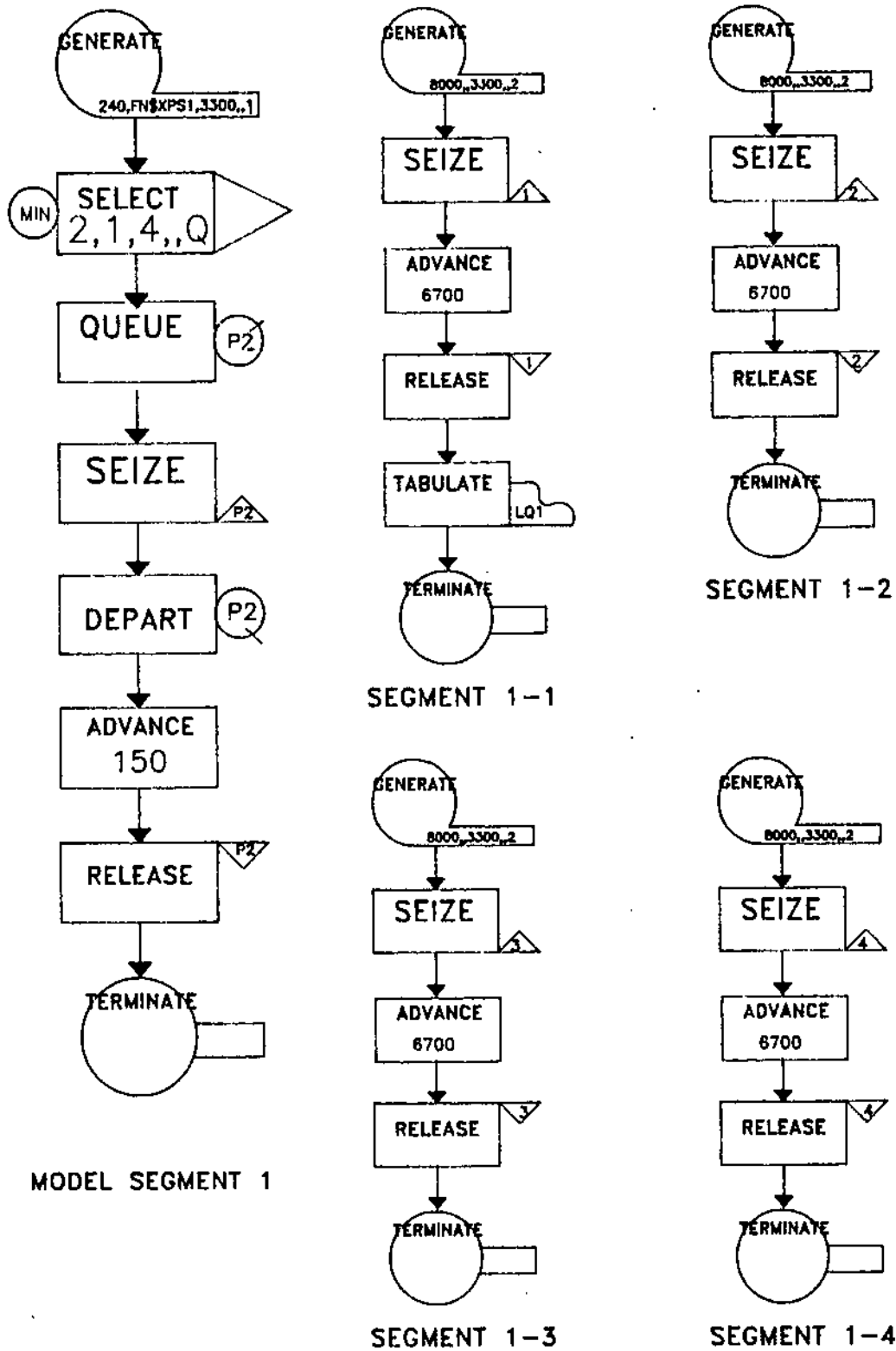
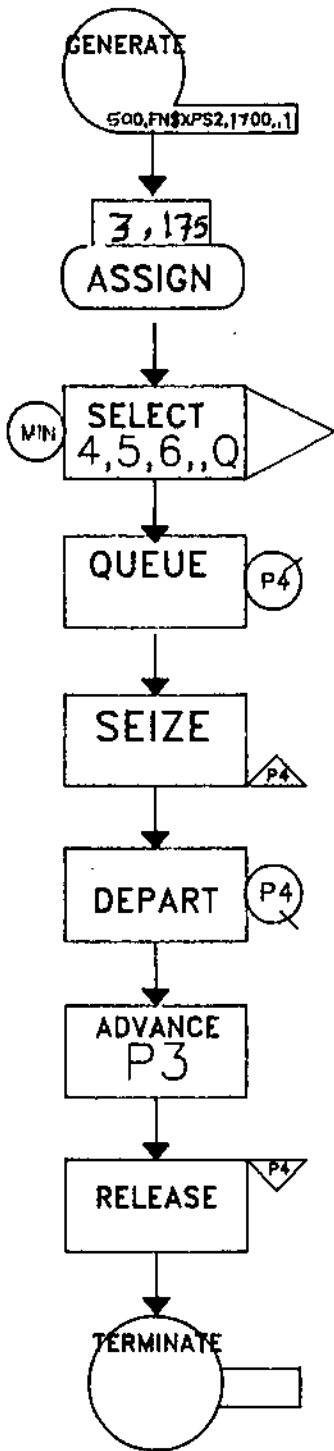
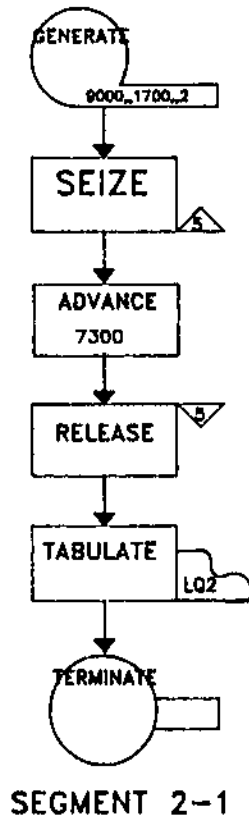


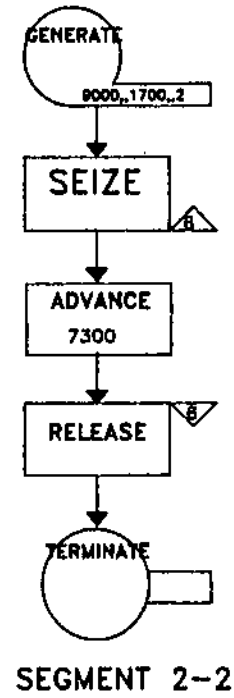
FIGURE 5.1 BLOCK DIAGRAM OF THE MODEL



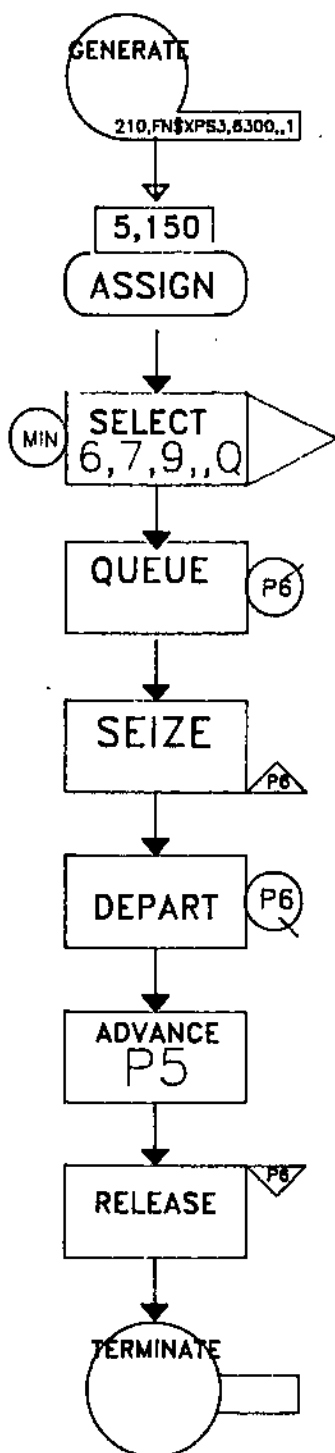
MODEL SEGMENT 2



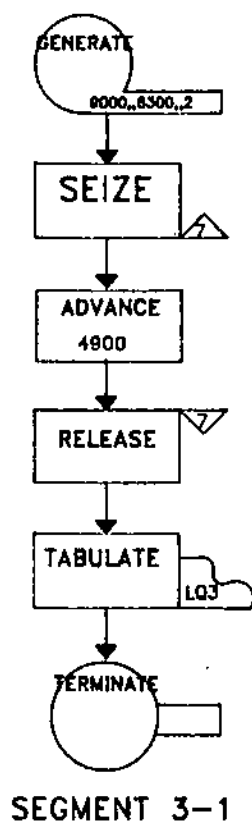
SEGMENT 2-1



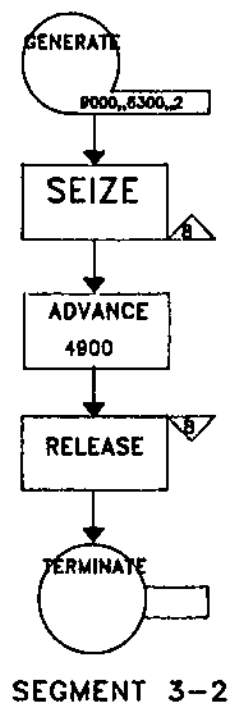
SEGMENT 2-2



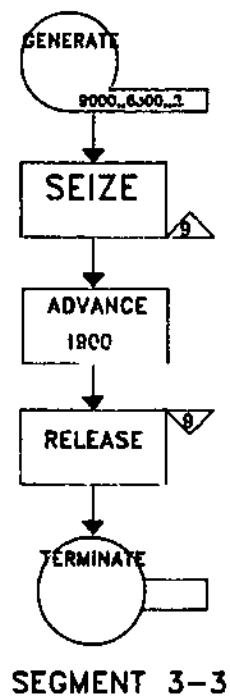
MODEL SEGMENT 3



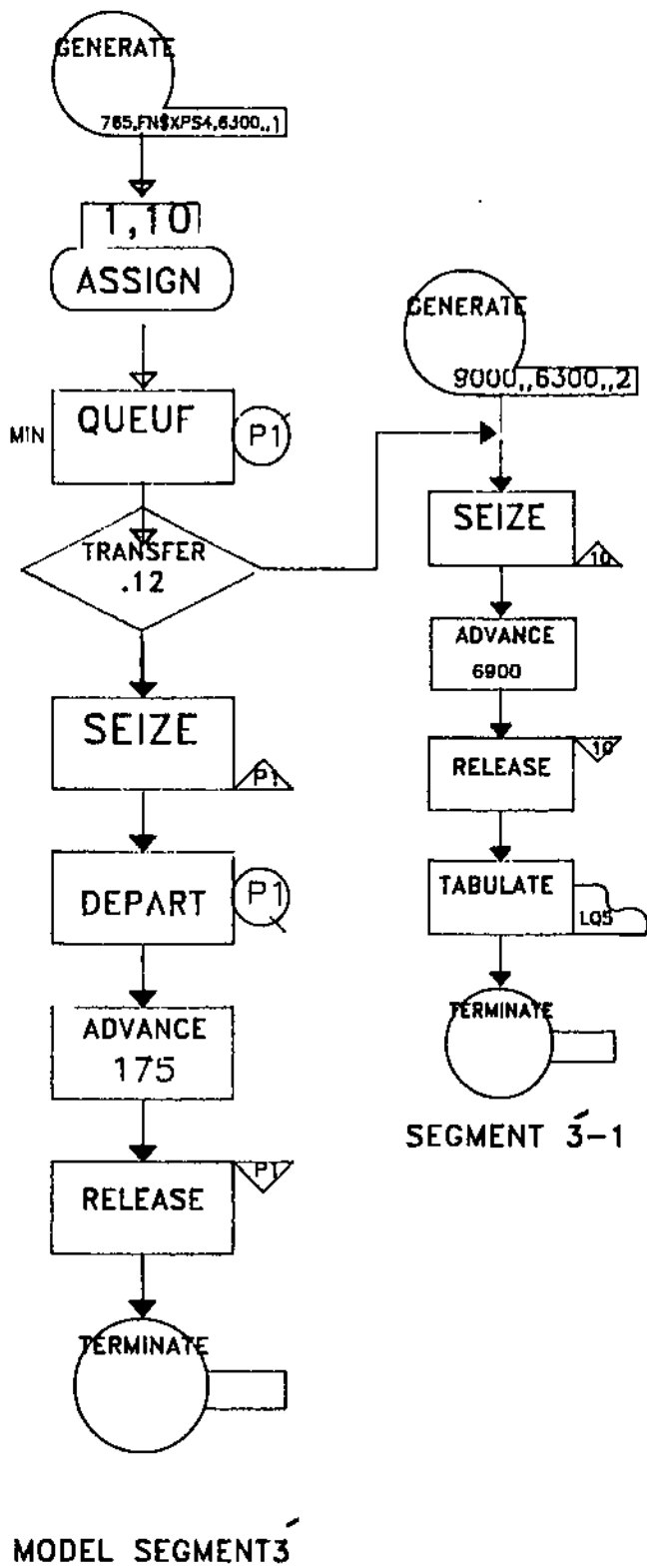
SEGMENT 3-1

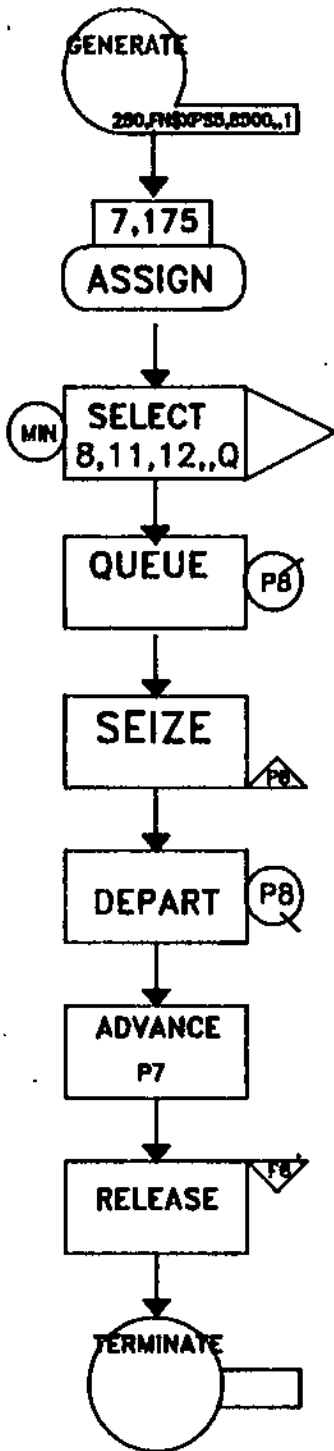


SEGMENT 3-2

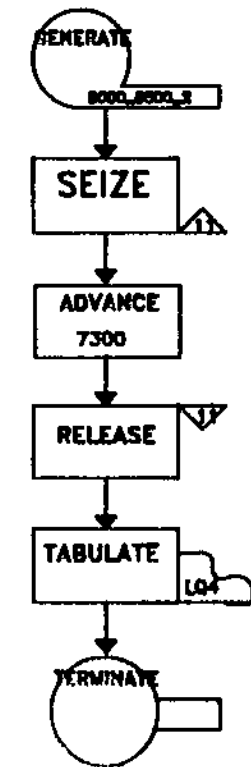


SEGMENT 3-3

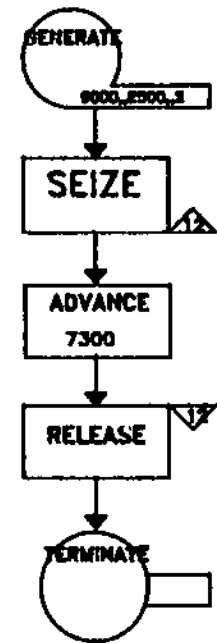




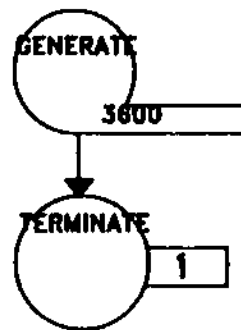
MODEL SEGMENT 4



SEGMENT 4-1



SEGMENT 4-2



MODEL SEGMENT 5

```

LINE#  SPMT#  IF DO  BLOCK#  *LOC  OPERATION      A,B,C,D,E,F,G  COMMENTS
-----
1      1
2      2          SIMULATE
3      3          RNULT      511,67,9,669,435
4      4          XPDS1 FUNCTION  RN1,C24
5      5          0,0/.1,.104/.2,.222/.3,.355/.4,.509/.5,.69/.6,.915/.7,1.2/.75,1.38
6      6          .8,1.6/.84,1.83/.88,2.12/.9,2.3/.92,2.52/.94,2.81/.95,2.99/.96,3.2
7      7          .97,3.5/.98,3.9/.99,4.6/.995,5.3/.998,6.2/.999,7/.9998,8
8      8          *****
9      9          XPDS2 FUNCTION  RN2,C24
10     10         0,0/.1,.104/.2,.222/.3,.355/.4,.509/.5,.69/.6,.915/.7,1.2/.75,1.38
11     11         .8,1.6/.84,1.83/.88,2.12/.9,2.3/.92,2.52/.94,2.81/.95,2.99/.96,3.2
12     12         .97,3.5/.98,3.9/.99,4.6/.995,5.3/.998,6.2/.999,7/.9998,8
13     13         *****
14     14         XPDS3 FUNCTION  RN3,C24
15     15         0,0/.1,.104/.2,.222/.3,.355/.4,.509/.5,.69/.6,.915/.7,1.2/.75,1.38
16     16         .8,1.6/.84,1.83/.88,2.12/.9,2.3/.92,2.52/.94,2.81/.95,2.99/.96,3.2
17     17         .97,3.5/.98,3.9/.99,4.6/.995,5.3/.998,6.2/.999,7/.9998,8
18     18         *****
19     19         XPDS4 FUNCTION  RN4,C24
20     20         0,0/.1,.104/.2,.222/.3,.355/.4,.509/.5,.69/.6,.915/.7,1.2/.75,1.38
21     21         .8,1.6/.84,1.83/.88,2.12/.9,2.3/.92,2.52/.94,2.81/.95,2.99/.96,3.2
22     22         .97,3.5/.98,3.9/.99,4.6/.995,5.3/.998,6.2/.999,7/.9998,8
23     23         *****
24     24         XPDS5 FUNCTION  RN5,C24
25     25         0,0/.1,.104/.2,.222/.3,.355/.4,.509/.5,.69/.6,.915/.7,1.2/.75,1.38
26     26         .8,1.6/.84,1.83/.88,2.12/.9,2.3/.92,2.52/.94,2.81/.95,2.99/.96,3.2
27     27         .97,3.5/.98,3.9/.99,4.6/.995,5.3/.998,6.2/.999,7/.9998,8
28     28
29     29          REALLOCATE  COM,1500000,FAC,900,QUE,900,CHA,900
30     30          LQ1 TABLE  Q1,0,1,50
31     31          LQ2 TABLE  Q5,0,1,50
32     32          LQ3 TABLE  Q7,0,1,50
33     33          LQ4 TABLE  Q11,0,1,50
34     34          LQ5 TABLE  Q10,0,1,50
35     35          1 GENERATE  230, FN$XPDS1,8000,,1
36     36          2 SELECT MIN  2,1,4,,Q
37     37          3 QUEUE      P2
38     38          4 SEIZE     P2
39     39          5 DEPART    P2
40     40          6 ADVANCE   150
41     41          7 RELEASE   P2
42     42          8 TERMINATE
43     43          *
44     44          9 AA GENERATE  11500,,8000,,2
45     45          10 SEIZE     1
46     46          11 ADVANCE   9500
47     47          12 RELEASE   1
48     48          13 TABULATE  LQ1
49     49          14 TERMINATE
50     50          15 AA1 GENERATE  11500,,8000,,2
51     51          16 SEIZE     2
52     52          17 ADVANCE   9500
53     53          18 RELEASE   2
54     54          19 TERMINATE
55     55          20 AA2 GENERATE  11500,,8000,,2
56     56          21 SEIZE     3
57     57          22 ADVANCE   9500

```

FIGURE 5.2 THE PROGRAM FOR THE MODEL USING GPSS LANG

GPSS/H VAX/VMS RELEASE 2.0-E (0Y088) 29 JAN 1991 14:34:35 FILE: 6FIRST.GPS

LINE#	STMT#	IF DO	BLOCK#	*LOC	OPERATION	A,B,C,D,E,F,G	COMMENTS
57	57		23		RELEASE	3	
58	58		24		TERMINATE		
59	59		25	AA3	GENERATE	11500,,8000,,2	
60	60		26		SEIZE	4	
61	61		27		ADVANCE	9500	
62	62		28		RELEASE	4	
63	63		29		TERMINATE		
64	64				*****		
65	65		30		GENERATE	500,FR\$XPDS2,2300,,1	
66	66		31		ASSIGN	3,175	
67	67		32		SELECT MIN	4,5,6,,0	
68	68		33		QUEUE	P4	
69	69		34		SEIZE	P4	
70	70		35		DEPART	P4	
71	71		36		ADVANCE	P3	
72	72		37		RELEASE	P4	
73	73		38		TERMINATE		
74	74			*			
75	75		39	BB	GENERATE	11500,,2300,,2	
76	76		40		SEIZE	5	
77	77		41	B	ADVANCE	9200	
78	78		42		RELEASE	5	
79	79		43		TABULATE	LQ2	
80	80		44		TERMINATE		
81	81		45	BB1	GENERATE	11500,,2300,,2	
82	82		46		SEIZE	6	
83	83		47	B1	ADVANCE	9200	
84	84		48		RELEASE	6	
85	85		49		TERMINATE		
86	86				*****		
87	87		50		GENERATE	210,FR\$XPDS3,8100,,1	
88	88		51		ASSIGN	5,150	
89	89		52		SELECT MIN	6,7,9,,0	
90	90		53		QUEUE	P6	
91	91		54		SEIZE	P6	
92	92		55		DEPART	P6	
93	93		56		ADVANCE	P5	
94	94		57		RELEASE	P6	
95	95		58		TERMINATE		
96	96			*			
97	97		59	HH1	GENERATE	11500,,8100,,2	
98	98		60		SEIZE	7	
99	99		61	H1	ADVANCE	6200	
100	100		62		RELEASE	7	
101	101		63		TABULATE	LQ3	
102	102		64		TERMINATE		
103	103		65	HH2	GENERATE	11500,,8100,,2	
104	104		66		SEIZE	8	
105	105		67	H2	ADVANCE	6200	
106	106		68		RELEASE	8	
107	107		69		TERMINATE		
108	108		70	HH3	GENERATE	11500,,8100,,2	
109	109		71		SEIZE	9	
110	110		72		ADVANCE	6200	
111	111		73		RELEASE	9	
112	112		74		TERMINATE		

GPSS/H VAX/VMS RELEASE 2.0-E (OV088) 29 JAN 1991 14:34:35 FILE: 6FIRST.GPS

LINE#	STMT#	IF DO	BLOCK#	*LOC	OPERATION	A,B,C,D,E,F,G	COMMENTS
113	113				*****		
114	114		75		GENERATE	765,FW\$XPDS4,5300,,1	
115	115		76		ASSIGN	1,10	
116	116		77		QUEUE	P1	
117	117		78		TRANSFER	.12,,UT	
118	118		79		SEIZE	P1	
119	119		80		DEPART	P1	
120	120		81		ADVANCE	175	
121	121		82		RELEASE	P1	
122	122		83		TERMINATE		
123	123		84	UT	SEIZE	P1	
124	124		85		DEPART	P1	
125	125		86		ADVANCE	275	
126	126		87		RELEASE	P1	
127	127		88		TERMINATE		
128	128			*			
129	129		89		GENERATE	11500,,5300,,2	
130	130		90		SEIZE	10	
131	131		91		ADVANCE	8700	
132	132		92		RELEASE	10	
133	133		93		TABULATE	LQ5	
131	134		94		TERMINATE		
135	135				*****		
136	136		95		GENERATE	460,FR\$XPDS5,11000,,1	
137	137		96		ASSIGN	7,175	
138	138		97		SELECT MIN	8,11,12,,Q	
139	139		98		QUEUE	P8	
140	140		99		SEIZE	P8	
141	141		100		DEPART	P8	
142	142		101		ADVANCE	P7	
143	143		102		RELEASE	P8	
144	144		103		TERMINATE		
145	145		104	DD	GENERATE	11500,,11000,,2	
146	146		105		SEIZE	11	
147	147		106		ADVANCE	9100	
148	148		107		RELEASE	11	
149	149		108		TABULATE	LQ4	
150	150		109		TERMINATE		
151	151		110	DD1	GENERATE	11500,,11000,,2	
152	152		111		SEIZE	12	
153	153		112		ADVANCE	9100	
154	154		113		RELEASE	12	
155	155		114		TERMINATE		
156	156				*****		
157	157		115	GG	GENERATE	360000	
158	158		116		TERMINATE	100	
159	159				START	1	
160	160				RMULT	741,543,789,771,121	
161	161				CLEAR		
162	162				START	1	
163	163				RMULT	123,511,657,287,191	
164	164				CLEAR		
165	165				START	1	
166	166				RMULT	87,991,733,655,21	
167	167				CLEAR		
168	168				START	1	

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LINE#	STMT#	IF DO	BLOCK#	*LOC	OPERATION	A,B,C,D,E,F,G	COMMENTS
169	169				RMULT	565,989,787,33,7	
170	170				CLEAR		
171	171				START	1	
172	172				RMULT	11,121,,345,569,23	
173	173				CLEAR		
174	174				START	1	
175	175				RMULT	651,537,449,373,111	
176	176				CLEAR		
177	177				START	1	
178	178				END		

in each queue, and the zero entries (the transactions that will wait zero time in the queue).

3- Tables contain the distribution of the queue content i.e. the queue content that stayed in the queue for certain percentage of time, and also the most likely queue content occurred. This value is helpful in determining the performance of the system better than the maximum value of the queue content.

CHAPTER SIX

IMPLEMENTATION AND RESULTS

All the data prepared for the existing situation was applied to the developed computer program, the program run perfectly, and the results were obtained, analysed, and tabulated.

6.1 VALIDATION :

Comparing the performance measures output by the simulation model to the equivalent performance measures taken from the real system, is the most often suggested method of validation a simulation model. The results of the program when the current data was applied to it, and the current situation were compared with each other. Table 6.1 shows this comparison.

Chi-square test will be used to test the goodness of fit for the results in table 6.1. Let O_1, O_2, \dots, O_n , observed frequencies of the maximum queue content, and E_1, E_2, \dots, E_k is the simulated results frequency of maximum queue content. Then the following hypothesis will be tested:

H_0 : Observed frequency for maximum queue content is the same as frequency results from simulation for maximum

TABLE 6.1 COMPARISON BETWEEN ACTUAL AND MODEL RESULTS

<i>Br. No.</i>	<i>REAL SYSTEM</i>		<i>MODEL SOLUTION</i>	
	Max queue content	Total entry to the system	Max queue content	Total entry to the system
<i>Br.1</i>	14	1500	13	1496
<i>Br.2</i>	18	720	17	708
<i>Br.3</i>	14	1670	13	1635
<i>Br.3</i>	20	470	21	448
<i>Br.4</i>	28	783	27	765

queue content.

H_1 : Observed frequency for maximum queue content is not the same as frequency results from simulation for maximum queue content.

We will test :

$$\chi^2_o = \sum_{i=1}^k \frac{(O_i - E_i)^2}{E_i} \dots\dots\dots 6.1$$

χ^2_o approximately follows the Chi-square distribution with $k-p-1$ degrees of freedom. Where p is the number of parameters of hypothesized distribution estimated by sample statistics.

The null hypothesis will be rejected if :

$$\chi^2_o > \chi^2_{\alpha, k-p-1}$$

The data in the following table will be tested :

Sample No.	O_i	E_i	$O_i - E_i$	$(O_i - E_i)^2 / E_i$
1	12	13	-1	1/13
2	14	13	1	1/13
3	13	12	-1	1/12
4	12	14	-2	4/14
5	15	14	1	1/14
6	13	13	0	0
7	14	12	2	4/12

By applying the data in the above table for equation 6.1,

$$\chi_0^2 = 0.93$$

and by taking the $\alpha = 0.05$, and from the tables of the Chi-square we can find that,

$$\chi_{0.05,6}^2 = 12.59$$

Since $\chi_0^2 < \chi_{0.05,6}^2$ so we can't reject the null hypothesis and we conclude that the observed frequency of the maximum queue content is the same as the resulted frequency of the maximum queue content taken from the simulation program.

6.2 ALTERNATE SOLUTIONS

Three alternate solutions will be studied.

6.2.1 FIRST ALTERNATE SOLUTION:

In this solution the best cycle time will be assigned based on the best combination for green time, while the cycle time is fixed.

In order to find the best cycle time for this intersection, first, we will find the best combination of green time to each branch, while fixing the cycle time.

Six combinations will be used for this study, as shown in table 6.2. For each combination, seven runs of the computer program were made, and the results were obtained.

All the results are analyzed and studied for each run, and tabulated in tables, as shown in tables 6.3 to 6.9.

TABLE 6.2 GREEN TIME FOR EACH BRANCH FOR DIFFERENT COMBINATIONS.

BRANCH NO.	COMBINATION NO.					
	<i>I</i>	<i>II</i>	<i>III</i>	<i>IV</i>	<i>V</i>	<i>VI</i>
<i>1</i>	25	20	15	20	20	20
<i>2</i>	20	25	30	30	25	23
<i>3</i>	60	50	40	45	45	53
<i>3'</i>	30	25	20	20	20	28
<i>4</i>	20	25	30	25	30	24

TABLE 6.3 SUMMARY OF OUTPUT OF COMBINATION NO. I

Run No.	MAX. QUEUE CONTENT				QUEUE CONTENT (MOST LIKELY OCCURRED)			
	BR.1	BR.2	BR.3	BR.4	BR.1	BR.2	BR.3	BR.4
1.	13	18	13	19	27	9	13	18
2.	14	17	13	22	27	10	11	20
3.	13	16	12	21	27	10	11	17
4.	12	17	12	21	28	9	9	20
5.	13	18	13	20	27	9	11	19
6.	13	16	13	22	24	10	13	21
7.	14	18	13	20	26	10	12	20
AV.	13	17	13	21	27	10	11	19

TABLE 6.4 SUMMARY OF OUTPUT OF COMBINATION NO. II

Run No.	MAX. QUEUE CONTENT				QUEUE CONTENT (MOST LIKELY OCCURRED)				
	BR.1	BR.2	BR.3	BR.3	BR.1	BR.2	BR.3	BR.3	BR.4
1.	14	13	14	37	13	13	11	11	10
2.	14	13	14	34	13	13	10	20	10
3.	13	13	12	46	14	9	10	23	11
4.	13	14	14	34	12	10	10	22	10
5.	14	14	14	45	13	10	10	28	10
6.	12	12	13	58	14	9	11	30	11
7.	13	12	13	58	12	9	10	31	10
av.	13	13	13	43	13	10	10	25	10

TABLE 6.5 SUMMARY OF OUTPUT OF COMBINATION NO. III

RUN NO.	MAX. QUEUE CONTENT				QUEUE CONTENT (MOST LIKELY OCCURRED)				
	BR.1	BR.2	BR.3	BR.3	BR.1	BR.2	BR.3	BR.3	BR.4
1.	92	11	16	104	12	12	12	62	9
2.	84	12	16	96	12	9	12	47	9
3.	69	12	14	120	13	8	12	59	9
4.	74	14	16	111	11	9	12	63	10
5.	90	14	16	104	13	9	11	62	11
6.	81	11	15	126	13	8	12	62	10
7.	78	11	15	124	12	8	11	62	9
AV.	83	12	15	109	12	9	12	60	10

TABLE 6.6 SUMMARY OF OUTPUT OF COMBINATION NO. IV

RUN NO.	MAX. QUEUE CONTENT				QUEUE CONTENT (MOST LIKELY OCCURRED)				
	BR.1	BR.2	BR.3	BR.4	BR.1	BR.2	BR.3	BR.4	
1.	14	12	15	109	11	9	11	67	10
2.	15	13	16	103	11	9	11	52	10
3.	13	12	13	120	10	8	11	59	10
4.	13	14	15	104	11	9	11	61	9
5.	14	14	15	107	11	9	11	62	10
6.	13	12	15	132	11	9	11	62	10
7.	14	12	14	130	11	9	12	61	10
AV.	14	13	15	117	11	9	11	60	10

TABLE 6.7 SUMMARY OF OUTPUT OF COMBINATION NO. 0

Run No.	MAX. QUEUE CONTENT				QUEUE CONTENT (MOST LIKELY OCCURRED)				
	BR.1	BR.2	BR.3	BR.4	BR.1	BR.2	BR.3	BR.4	
1.	14	13	16	109	11	11	11	67	9
2.	14	13	15	103	11	9	11	52	9
3.	13	13	13	120	10	10	11	59	9
4.	14	14	15	104	11	10	11	61	11
5.	14	14	15	109	11	10	11	62	11
6.	12	12	15	130	11	9	11	62	10
7.	13	12	14	124	11	9	11	62	9
AV.	13	13	15	113	11	10	11	61	10

TABLE 6.0 SUMMARY OF OUTPUT OF COMBINATION NO. VI
 MAX. QUEUE CONTENT

Run No.	MAX. QUEUE CONTENT								QUEUE CONTENT (MOST LIKELY OCCURRED)			
	BR.1	BR.2	BR.3	BR.3 ⁻	BR.4	BR.1	BR.2	BR.3	BR.3 ⁻	BR.4		
1.	13	13	14	26	14	10	10	10	15	10		
2.	14	14	14	26	14	10	10	10	15	10		
3.	13	13	14	23	13	10	10	10	13	10		
4.	14	14	12	27	13	9	10	10	14	11		
5.	14	14	14	23	13	10	10	10	14	10		
6.	12	15	14	26	13	9	10	16	16	11		
7.	16	13	14	29	14	11	10	17	11	10		
NO.	14	14	13	26	14	10	10	15	10	10		

By analysing these results it can be shown that combination number VI is the best one. This can be decided by considering the following two points:

1- Minimize the maximum queue content.

2- Minimize the current queue content which is the most likely occurred.

Table 6.10 shows the best combination of the green time for all branches. This combination will be used in studying the best cycle time.

To find the best cycle time, various values of cycle time will be studied; 80, 90, 100, 110, and 120 seconds. Table 6.10 shows the green time for each branch in each cycle time based on the best combination of the green time.

For each cycle time, seven runs of the computer program were performed, and the results were obtained. All the results were analyzed and studied for each run, and tabulated in tables, as shown in tables 6.11 to 6.15.

By analysing the results, it can be shown that the best cycle time is 90 seconds, based on the above two points mentioned above. Figure 6.1 shows different cycle times versus queue content.

To predict the results for the future, the forecasted data of the traffic flow for year 2000 is applied to the

TABLE 6.9 BEST COMBINATION OF GREEN TIME.

Br. No.	1	2	3	3'	4
G.T.	20	23	53	28	24

TABLE 6.10 GREEN TIME FOR EACH BRANCH FOR DIFFERENT CYCLE TIMES

CYCLE TIME.	GREEN TIME FOR BRANCH No.				
	<i>1</i>	<i>2</i>	<i>3</i>	<i>3'</i>	<i>4</i>
80	13	15	36	18	14
90	15	17	41	21	17
100	17	20	45	23	20
110	19	22	50	26	23
120	21	25	55	29	25

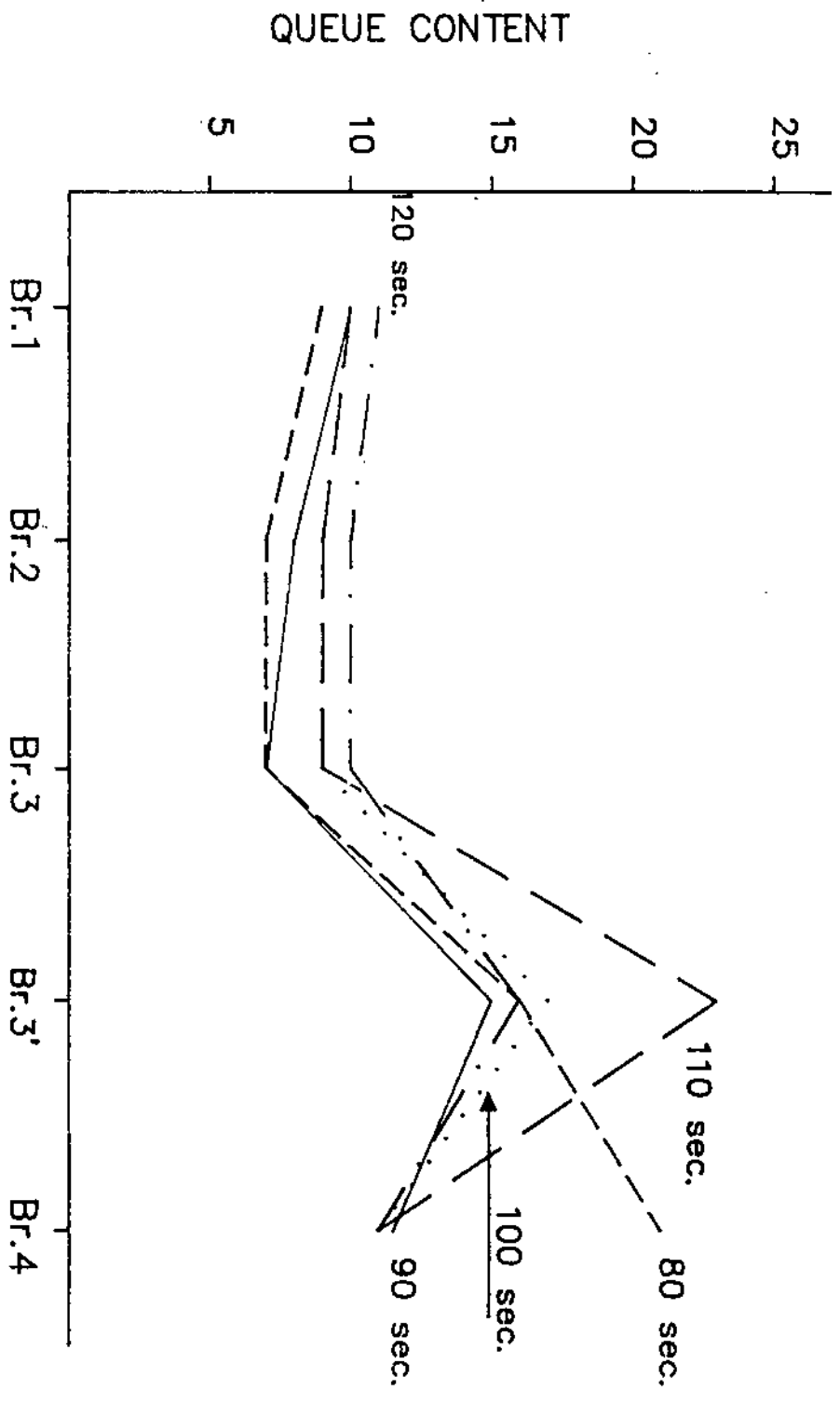


FIGURE 6.1 QUEUE DISTRIBUTION IN EACH BRANCH FOR DIFFERENT CYCLE TIMES

TABLE 6.11 SUMMARY OF OUTPUT OF THE PROGRAM WITH CYCLE TIME = 98 sec.

RUN NO.	MAX. QUEUE CONTENT				(MOST LIKELY OCCURRED)					
	BR.1	BR.2	BR.3	BR.3	BR.4	BR.1	BR.2	BR.3	BR.3	BR.4
1.	12	11	11	26	30	9	8	7	13	21
2.	13	11	9	18	36	9	7	7	11	22
3.	11	11	8	33	33	9	8	6	16	19
4.	12	14	9	25	35	9	7	7	14	21
5.	13	13	10	35	35	9	7	7	21	21
6.	11	11	10	40	27	9	8	7	20	19
7.	13	11	9	39	39	9	7	7	20	23
AV.	12	12	9	31	33	9	7	7	16	21

TABLE 6.12 SUMMARY OF OUTPUT OF THE PROGRAM WITH CYCLE TIME = 98 sec.

RUN NO.	MAX. QUEUE CONTENT				QUEUE CONTENT (MOST LIKELY OCCURRED)				
	BR.1	BR.2	BR.3	BR.3	BR.1	BR.2	BR.3	BR.3	BR.4
1.	15	12	12	21	10	9	7	12	11
2.	13	12	11	19	10	8	7	11	11
3.	14	12	12	31	10	9	8	15	11
4.	13	12	9	26	9	8	7	14	11
5.	14	13	11	34	10	8	7	20	11
6.	13	11	10	28	11	9	8	17	11
7.	13	12	11	33	9	8	7	19	10
AV.	14	12	11	27	10	8	7	15	11

TABLE 6.13 SUMMARY OF OUTPUT OF THE PROGRAM WITH CYCLE TIME = 100 sec.

RUN NO.	MAX. QUEUE CONTENT					QUEUE CONTENT (MOST LIKELY OCCURRED)				
	BR.1	BR.2	BR.3	BR.3	BR.4	BR.1	BR.2	BR.3	BR.3	BR.4
1.	11	12	12	33	14	10	9	8	16	10
2.	13	12	12	21	13	10	8	9	12	10
3.	14	12	12	36	13	10	9	9	18	10
4.	12	13	11	27	13	10	9	8	15	10
5.	14	13	12	31	12	10	9	8	18	10
6.	11	12	12	38	14	9	9	9	20	9
7.	12	11	13	37	12	10	8	9	20	10
AV.	12	12	12	32	13	10	9	9	17	10

TABLE 6.14 SUMMARY OF OUTPUT OF THE PROGRAM WITH CYCLE TIME = 110 sec.

RUN NO.	MAX. QUEUE CONTENT				QUEUE CONTENT (MOST LIKELY OCCURRED)						
	BR.1	BR.2	BR.3	BR.4	BR.1	BR.2	BR.3	BR.4			
1.	14	13	13	31	13	13	11	10	9	19	9
2.	14	12	12	30	13	13	11	9	9	18	10
3.	13	12	12	52	13	13	10	9	9	22	10
4.	12	15	12	31	14	14	10	9	9	21	10
5.	14	14	12	43	13	13	10	9	9	28	10
6.	14	12	13	49	14	14	10	9	9	24	10
7.	14	13	12	51	13	13	10	9	9	29	10
AV.	14	13	12	42	13	13	10	9	9	23	10

TABLE 6.15 SUMMARY OF OUTPUT OF THE PROGRAM WITH CYCLE TIME = 120 sec.

RUN NO.	MAX. QUEUE CONTENT						QUEUE CONTENT (MOST LIKELY OCCURRED)					
	BR.1	BR.2	BR.3	BR.3	BR.4		BR.1	BR.2	BR.3	BR.3	BR.4	
1.	15	13	14	26	14		11	10	10	15	10	
2.	15	16	13	19	13		11	10	10	12	11	
3.	16	14	14	28	14		12	10	11	16	11	
4.	14	14	13	23	13		11	10	10	14	10	
5.	15	16	13	25	14		11	10	10	17	11	
6.	13	13	14	25	15		11	10	10	17	11	
7.	15	13	14	31	14		11	10	10	20	11	
AV.	15	14	14	25	14		11	10	10	16	11	

TABLE 6.16 SUMMARY OF OUTPUT OF THE PROGRAM FOR ALTERNATE SOLUTION 1.

RUN NO.	MAX. QUEUE CONTENT				QUEUE CONTENT (MOST LIKELY OCCURED)					
	BR.1	BR.2	BR.3	BR.3	BR.1	BR.2	BR.3	BR.3	BR.4	
1.	244	97	16	259	389	118	57	11	138	158
2.	218	96	15	244	305	112	53	11	119	161
3.	220	99	16	255	382	112	56	11	134	158
4.	218	188	17	253	303	112	53	11	135	159
5.	214	109	15	273	304	110	54	11	140	161
6.	231	94	14	248	318	119	56	11	131	162
7.	213	96	16	247	309	108	54	11	134	163
AV.	220	100	16	254	307	113	55	11	133	160

TABLE 6.17 SUMMARY OF OUTPUT OF THE PROGRAM FOR ALTERNATE SOLUTION 2.

RUN NO.	MAX. QUEUE CONTENT				QUEUE CONTENT (MOST LIKELY OCCURRED)				
	BR.1	BR.2	BR.3	BR.3	BR.1	BR.2	BR.3	BR.3	BR.4
1.	79	19	15	233	43	13	9	123	50
2.	73	15	14	225	40	11	10	110	52
3.	72	19	13	273	37	12	9	142	53
4.	80	22	14	229	43	12	9	123	50
5.	70	22	14	253	40	12	9	127	52
6.	77	17	12	235	41	12	9	125	52
7.	77	18	12	266	40	12	9	136	53
AV.	76	19	13	245	41	12	9	126	52

LINE#	STMT#	IF DO	BLOCK#	*LOC	OPERATION	A,B,C,D,E,F,G	COMMENTS
1	1				SIMULATE		
2	2				RMULT	511,67,9,669,435	
3	3				XPDS1 FUNCTION	RN1,C24	
4	4					0,0/.1,.104/.2,.222/.3,.355/.4,.509/.5,.69/.6,.915/.7,1.2/.75,1.38	
5	5					.8,1.6/.84,1.83/.88,2.12/.9,2.3/.92,2.52/.94,2.81/.95,2.99/.96,3.2	
6	6					.97,3.5/.98,3.9/.99,4.6/.995,5.3/.998,6.2/.999,7/.9998,8	
7	7					*****	
8	8				XPDS2 FUNCTION	RN2,C24	
9	9					0,0/.1,.104/.2,.222/.3,.355/.4,.509/.5,.69/.6,.915/.7,1.2/.75,1.38	
10	10					.8,1.6/.84,1.83/.88,2.12/.9,2.3/.92,2.52/.94,2.81/.95,2.99/.96,3.2	
11	11					.97,3.5/.98,3.9/.99,4.6/.995,5.3/.998,6.2/.999,7/.9998,8	
12	12					*****	
13	13				XPDS3 FUNCTION	RN3,C24	
14	14					0,0/.1,.104/.2,.222/.3,.355/.4,.509/.5,.69/.6,.915/.7,1.2/.75,1.38	
15	15					.8,1.6/.84,1.83/.88,2.12/.9,2.3/.92,2.52/.94,2.81/.95,2.99/.96,3.2	
16	16					.97,3.5/.98,3.9/.99,4.6/.995,5.3/.998,6.2/.999,7/.9998,8	
17	17					*****	
18	18				XPDS4 FUNCTION	RN4,C24	
19	19					0,0/.1,.104/.2,.222/.3,.355/.4,.509/.5,.69/.6,.915/.7,1.2/.75,1.38	
20	20					.8,1.6/.84,1.83/.88,2.12/.9,2.3/.92,2.52/.94,2.81/.95,2.99/.96,3.2	
21	21					.97,3.5/.98,3.9/.99,4.6/.995,5.3/.998,6.2/.999,7/.9998,8	
22	22					*****	
23	23				XPDS5 FUNCTION	RN5,C24	
24	24					0,0/.1,.104/.2,.222/.3,.355/.4,.509/.5,.69/.6,.915/.7,1.2/.75,1.38	
25	25					.8,1.6/.84,1.83/.88,2.12/.9,2.3/.92,2.52/.94,2.81/.95,2.99/.96,3.2	
26	26					.97,3.5/.98,3.9/.99,4.6/.995,5.3/.998,6.2/.999,7/.9998,8	
27	27						
28	28				REALLOCATE	COM,1500000,FAC,900,QUE,900,CHA,900	
29	29				LQ1 TABLE	Q1,0,1,50	
30	30				LQ2 TABLE	Q5,0,1,50	
31	31				LQ3 TABLE	Q8,0,1,50	
32	32				LQ4 TABLE	Q11,0,1,50	
33	33				LQ5 TABLE	Q14,0,1,50	
34	34		1		GENERATE	145,FN\$XPDS1,4600,,1	
35	35		2		SELECT MIN	2,1,4,,0	
36	36		3		QUEUE	P2	
37	37		4		SEIZE	P2	
38	38		5		DEPART	P2	
39	39		6		ADVANCE	150	
40	40		7		RELEASE	P2	
41	41		8		TERMINATE		
42	42			*			
43	43		9	AA	GENERATE	9000,,4600,,2	
44	44		10		SEIZE	1	
45	45		11		ADVANCE	7000	
46	46		12		RELEASE	1	
47	47		13		TABULATE	LQ1	
48	48		14		TERMINATE		
49	49		15	AA1	GENERATE	9000,,4600,,2	
50	50		16		SEIZE	2	
51	51		17		ADVANCE	7000	
52	52		18		RELEASE	2	
53	53		19		TERMINATE		
54	54		20	AA2	GENERATE	9000,,4600,,2	
55	55		21		SEIZE	3	
56	56		22		ADVANCE	7000	

Fig 6.3

LINE#	STMT#	IF DO	BLOCK#	*LOC	OPERATION	A,B,C,D,E,F,G	COMMENTS
57	57		23		RELEASE	3	
58	58		24		TERMINATE		
59	59		25	AA3	GENERATE	9000,,4600,,2	
60	60		26		SEIZE	4	
61	61		27		ADVANCE	7000	
62	62		28		RELEASE	4	
63	63		29		TERMINATE		
64	64				*****		
65	65		30		GENERATE	360, FN\$XPDS2,1400,,1	
66	66		31		ASSIGN	3,175	
67	67		32		SELECT MIN	4,5,7,,Q	
68	68		33		QUEUE	P4	
69	69		34		SEIZE	P4	
70	70		35		DEPART	P4	
71	71		36		ADVANCE	P3	
72	72		37		RELEASE	P4	
73	73		38		TERMINATE		
74	74			*			
75	75		39	BB	GENERATE	9000,,1400,,2	
76	76		40		SEIZE	5	
77	77		41	B	ADVANCE	7600	
78	78		42		RELEASE	5	
79	79		43		TABULATE	LQ2	
80	80		44		TERMINATE		
81	81		45	BB1	GENERATE	9000,,1400,,2	
82	82		46		SEIZE	6	
83	83		47	B1	ADVANCE	7600	
84	84		48		RELEASE	6	
85	85		49		TERMINATE		
86	86		50		GENERATE	9000,,1400,,2	
87	87		51		SEIZE	7	
88	88		52		ADVANCE	7600	
89	89		53		RELEASE	7	
90	90		54		TERMINATE		
91	91				*****		
92	92		55		GENERATE	135, FN\$XPDS3,6600,,1	
93	93		56		ASSIGN	5,150	
94	94		57		SELECT MIN	6,8,10,,Q	
95	95		58		QUEUE	P6	
96	96		59		SEIZE	P6	
97	97		60		DEPART	P6	
98	98		61		ADVANCE	P5	
99	99		62		RELEASE	P6	
100	100		63		TERMINATE		
101	101			t			
102	102		64	HH1	GENERATE	9000,,6600,,2	
103	103		65		SEIZE	8	
104	104		66	H1	ADVANCE	4300	
105	105		67		RELEASE	8	
106	106		68		TABULATE	LQ3	
107	107		69		TERMINATE		
108	108		70	HH2	GENERATE	9000,,6600,,2	
109	109		71		SEIZE	9	
110	110		72	H2	ADVANCE	4300	
111	111		73		RELEASE	9	
112	112		74		TERMINATE		

LINE#	STMT#	IF	DO	BLOCK#	*LOC	OPERATION	A,B,C,D,E,F,G	COMMENTS
113	113			75	BH3	GENERATE	9000,,6600,,2	
114	114			76		SEIZE	10	
115	115			77		ADVANCE	4300	
116	116			78		RELEASE	10	
117	117			79		TERMINATE		
118	118					*****		
119	119			80		GENERATE	485,FN\$XPDS4,4100,,1	
120	120			81		ASSIGN	1,14	
121	121			82		QUEUE	P1	
122	122			83		TRANSFER	.12,,UT	
123	123			84		SEIZE	P1	
124	124			85		DEPART	P1	
125	125			86		ADVANCE	175	
126	126			87		RELEASE	P1	
127	127			88		TERMINATE		
128	128			89	UT	SEIZE	P1	
129	129			90		DEPART	P1	
130	130			91		ADVANCE	275	
131	131			92		RELEASE	P1	
132	132			93		TERMINATE		
133	133				*			
134	134			94		GENERATE	9000,,4100,,2	
135	135			95		SEIZE	14	
136	136			96		ADVANCE	6800	
137	137			97		RELEASE	14	
138	138			98		TABULATE	LQ5	
139	139			99		TERMINATE		
140	140					*****		
141	141			100		GENERATE	288,FN\$XPDS5,8500,,1	
142	142			101		ASSIGN	7,180	
143	143			102		SELECT MIN	8,11,13,,0	
144	144			103		QUEUE	P8	
145	145			104		SEIZE	P8	
146	146			105		DEPART	P8	
147	147			106		ADVANCE	P7	
148	148			107		RELEASE	P8	
149	149			108		TERMINATE		
150	150			109	DD	GENERATE	9000,,8500,,2	
151	151			110		SEIZE	11	
152	152			111		ADVANCE	7600	
153	153			112		RELEASE	11	
154	154			113		TABULATE	LQ4	
155	155			114		TERMINATE		
156	156			115	DD1	GENERATE	9000,,8500,,2	
157	157			116		SEIZE	12	
158	158			117		ADVANCE	7600	
159	159			118		RELEASE	12	
160	160			119		TERMINATE		
161	161			120		GENERATE	9000,,8500,,2	
162	162			121		SEIZE	13	
163	163			122		ADVANCE	7600	
164	164			123		RELEASE	13	
165	165			124		TERMINATE		
166	166					*****		
167	167			125	GG	GENERATE	360000	
168	168			126		TERMINATE	100	

LINE#	STMT#	IF DO	BLOCK#	*LOC	OPERATION	A,B,C,D,E,F,G	COMMENTS
169	169				START	1	
170	170				RMULT	741,543,789,771,121	
171	171				CLEAR		
172	172				START	1	
173	173				RMULT	123,51,657,87,91	
174	174				CLEAR		
175	175				START	1	
176	176				RMULT	87,991,733,655,21	
177	177				CLEAR		
178	178				START	1	
179	179				RMULT	565,989,787,33,7	
180	180				CLEAR		
181	181				START	1	
182	182				RMULT	11,121,,345,569,23	
183	183				CLEAR		
184	184				START	1	
185	185				RMULT	51,37,49,73.111	
186	186				CLEAR		
187	187				START	1	
188	188				END		

program using the best cycle time based on the best green time combination. seven runs were performed, the results were obtained, studied for each run, and tabulated, as shown in table 6.16. It is easily noticed that, there are very long queues for most of the branches, and this can't be handled by the existing intersection geometry.

6.2.2 SECOND ALTERNATE SOLUTION:

In this solution number of lanes in branch 2 and, 4 are to be increased up to three lanes for each branch. According to this change, the program also should be modified to accommodate with the new changes. Figure 6.3 shows the modified program that presents the modified model.

The forecasted traffic flow for year 2000 was applied to the modified program, and the program run well, and the results were obtained, and tabulated, as shown in table 6.17. By examining and analysing the results, one can notice that there are large queues in some branches specially in branches 1 and 3'. So this solution cannot help.

6.2.3 THIRD ALTERNATIVE

In this solution, underpass (tunnel) will be constructed to allow continues passing for the traffic flow.

The best position to construct the tunnel will be on the heaviest traffic flow branches, they are branch 1 and 3, this will be along the QUEEN NOOR STREET. In this case number of phases will be reduced, and more movements can be put in this intersection.

The program of the simulation model will be modified to accommodate the new changing in the intersection geometry, so lanes for passing through in branches 1 and 3 will be eliminated from the program. Figure 6.4 shows the new modified program which accommodated with the third alternate solution.

The forecasted flow counts for year 2000 is applied to the modified program, and the program was run, and the results were obtained and tabulated as shown in table 6.18. By analysing the results, in the table 6.18, it is easily noticed that there is no problem, and all the branches have reasonable and acceptance queue length, or queue content at any time specially during the rush hour.

So the third alternate solution will be an acceptable solution for the intersection regardless the cost.

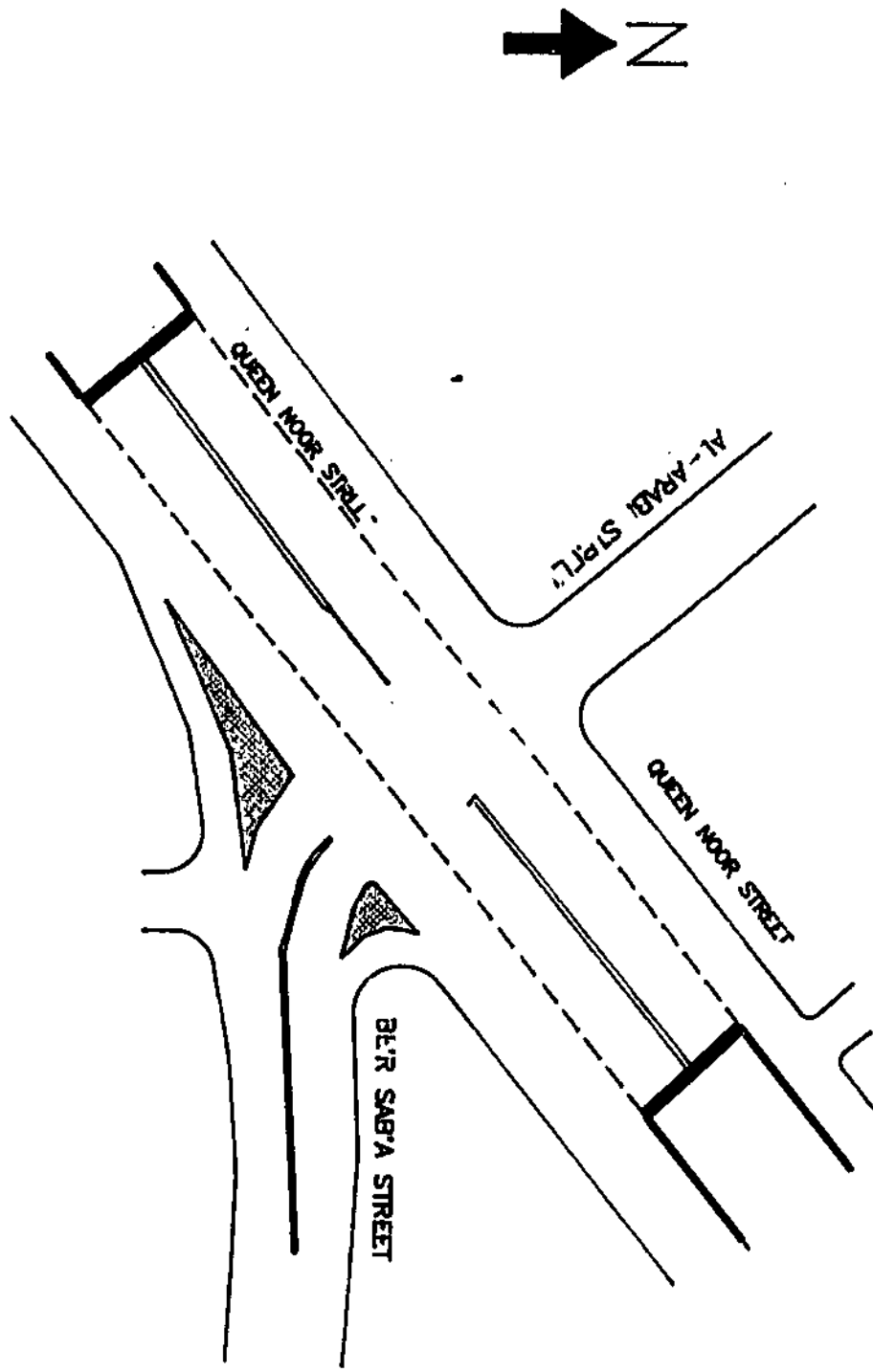


FIGURE 6.2 THE INTERSECTION GEOMETRY WHEN APPLYING THE THIRD ALTERNATE SOLUTION.

LINE#	STMT#	IF	DD	BLOCK#	*LOC	OPERATION	A,B,C,D,E,F,G	COMMENTS
1	1					SIMULATE		
2	2					RMULT	511,67,9,669,435	
3	3					XPDS1 FUNCTION	RN1,C24	
4	4						0,0/.1,.104/.2,.222/.3,.355/.4,.509/.5,.69/.6,.915/.7,1.2/.75,1.38	
5	5						.8,1.6/.84,1.83/.88,2.12/.9,2.3/.92,2.52/.94,2.81/.95,2.99/.96,3.2	
6	6						.97,3.5/.98,3.9/.99,4.6/.995,5.3/.998,6.2/.999,7/.9998,8	
7	7					*****		
8	8					XPDS2 FUNCTION	RN2,C24	
9	9						0,0/.1,.104/.2,.222/.3,.355/.4,.509/.5,.69/.6,.915/.7,1.2/.75,1.38	
10	10						.8,1.6/.84,1.83/.88,2.12/.9,2.3/.92,2.52/.94,2.81/.95,2.99/.96,3.2	
11	11						.97,3.5/.98,3.9/.99,4.6/.995,5.3/.998,6.2/.999,7/.9998,8	
12	12					*****		
13	13					XPDS3 FUNCTION	RN3,C24	
14	14						0,0/.1,.104/.2,.222/.3,.355/.4,.509/.5,.69/.6,.915/.7,1.2/.75,1.38	
15	15						.8,1.6/.84,1.83/.88,2.12/.9,2.3/.92,2.52/.94,2.81/.95,2.99/.96,3.2	
16	16						.97,3.5/.98,3.9/.99,4.6/.995,5.3/.998,6.2/.999,7/.9998,8	
17	17					*****		
18	18					XPDS4 FUNCTION	RN4,C24	
19	19						0,0/.1,.104/.2,.222/.3,.355/.4,.509/.5,.69/.6,.915/.7,1.2/.75,1.38	
20	20						.8,1.6/.84,1.83/.88,2.12/.9,2.3/.92,2.52/.94,2.81/.95,2.99/.96,3.2	
21	21						.97,3.5/.98,3.9/.99,4.6/.995,5.3/.998,6.2/.999,7/.9998,8	
22	22					*****		
23	23					XPDS5 FUNCTION	RN5,C24	
24	24						0,0/.1,.104/.2,.222/.3,.355/.4,.509/.5,.69/.6,.915/.7,1.2/.75,1.38	
25	25						.8,1.6/.84,1.83/.88,2.12/.9,2.3/.92,2.52/.94,2.81/.95,2.99/.96,3.2	
26	26						.97,3.5/.98,3.9/.99,4.6/.995,5.3/.998,6.2/.999,7/.9998,8	
27	27							
28	28					REALLOCATE	COM,1500000,FAC,900,QUE,900,CHA,900	
29	29					LQ1 TABLE	Q1,0,1,50	
30	30					LQ2 TABLE	Q5,0,1,50	
31	31					LQ3 TABLE	Q7,0,1,50	
32	32					LQ4 TABLE	Q11,0,1,50	
33	33					LQ5 TABLE	Q7,0,1,50	
34	34				1	GENERATE	485,FN\$XPDS1,2000,,1	
35	35				2	ASSIGN	2,200	
36	36				3	SELECT MIN	1,1,2,,Q	
37	37				4	QUEUE	P1	
38	38				5	SEIZE	P1	
39	39				6	DEPART	P1	
40	40				7	ADVANCE	P2	
41	41				8	RELEASE	P1	
42	42				9	TERMINATE		
43	43				*			
44	44				10	GENERATE	9000,,2000,,2	
45	45				11	SEIZE	1	
46	46				12	ADVANCE	7000	
47	47				13	RELEASE	1	
48	48				14	TABULATE	LQ1	
49	49				15	TERMINATE		
50	50				*			
51	51				16	GENERATE	9000,,2000,,2	
52	52				17	SEIZE	2	
53	53				18	ADVANCE	7000	
54	54				19	RELEASE	2	
55	55				20	TERMINATE		
56	56				*			

Fig 6.4

LINE#	STMT#	IF DO	BLOCK#	*LOC	OPERATION	A,B,C,D,E,F,G	COMMENTS
57	57				*****		
58	58		21		GENERATE	360,FN\$XPDS2,5200,,1	
59	59		22		ASSIGN	3,175	
60	60		23		SELECT MIN	4,5,6,,Q	
61	61		24		QUEUE	P4	
62	62		25		SEIZE	P4	
63	63		26		DEPART	P4	
64	64		27		ADVANCE	P3	
65	65		28		RELEASE	P4	
66	66		29		TERMINATE		
67	67			*			
68	68		30	BB	GENERATE	9000,,5200,,2	
69	69		31		SEIZE	5	
70	70		32	B	ADVANCE	6300	
71	71		33		RELEASE	5	
72	72		34		TABULATE	LQ2	
73	73		35		TERMINATE		
74	74		36	BB1	GENERATE	9000,,5200,,2	
75	75		37		SEIZE	6	
76	76		38	B1	ADVANCE	6300	
77	77		39		RELEASE	6	
78	78		40		TERMINATE		
79	79				*****		
80	80		41		GENERATE	485,FN\$XPDS4,2000,,1	
81	81		42		SELECT MIN	5,7,8,,Q	
82	82		43		QUEUE	P5	
83	83		44		TRANSFER	.12,,UT	
84	84		45		SEIZE	P5	
85	85		46		DEPART	P5	
86	86		47		ADVANCE	170	
87	87		48		RELEASE	P5	
88	88		49		TERMINATE		
89	89		50	UT	SEIZE	P5	
90	90		51		DEPART	P5	
91	91		52		ADVANCE	275	
92	92		53		RELEASE	P5	
93	93		54		TERMINATE		
94	94			*			
95	95		55		GENERATE	9000,,2000,,2	
96	96		56		SEIZE	7	
97	97		57		ADVANCE	7000	
98	98		58		RELEASE	7	
99	99		59		TABULATE	LQ5	
100	100		60		TERMINATE		
101	101		61		GENERATE	9000,,2000,,2	
102	102		62		SEIZE	8	
103	103		63		ADVANCE	7000	
104	104		64		RELEASE	8	
105	105		65		TERMINATE		
106	106				*****		
107	107		66		GENERATE	288,FN\$XPDS5,8500,,1	
108	108		67		ASSIGN	7,175	
109	109		68		SELECT MIN	8,11,12,,Q	
110	110		69		QUEUE	P8	
111	111		70		SEIZE	P8	
112	112		71		DEPART	P8	

LINE#	STMT#	IF DO	BLOCK#	*LOC	OPERATION	A,B,C,D,E,F,G	COMMENTS
113	113		72		ADVANCE	P7	
114	114		73		RELEASE	P8	
115	115		74		TERMINATE		
116	116		75	DD	GENERATE	9000,,8500,,2	
117	117		76		SEIZE	11	
118	118		77		ADVANCE	6200	
119	119		78		RELEASE	11	
120	120		79		TABULATE	LQ4	
121	121		80		TERMINATE		
122	122		81	DD1	GENERATE	9000,,8500,,2	
123	123		82		SEIZE	12	
124	124		83		ADVANCE	6200	
125	125		84		RELEASE	12	
126	126		85		TERMINATE		
127	127				*****		
128	128		86	GG	GENERATE	360000	
129	129		87		TERMINATE	100	
130	130				START	1	
131	131				RMULT	741,543,789,771,121	
132	132				CLEAR		
133	133				START	1	
134	134				RMULT	123,51,657,87,91	
135	135				CLEAR		
136	136				START	1	
137	137				RMULT	87,991,733,655,21	
138	138				CLEAR		
139	139				START	1	
140	140				RMULT	565,989,787,33,7	
141	141				CLEAR		
142	142				START	1	
143	143				RMULT	11,121,,345,569,23	
144	144				CLEAR		
145	145				START	1	
146	146				RMULT	51,37,49,73,111	
147	147				CLEAR		
148	148				START	1	
149	149				END		

6.3 WHEN TO APPLY THE PROPOSED SOLUTION

In order to determine the best time to apply the third alternate solution, the following small study is applied: Since the best cycle time, based on best combination is determined, the forecasted traffic flow for years 1990, 1991,..... 2000, and check the results, and determine in which year that the intersection can't handle any more flow, because after that year there is no way to solve the problem of the queue length except by changing the geometry of the intersection, and this can be done by using the third alternate solution.

TABLE 6.18 SUMMARY OF OUTPUT OF THE PROGRAM FOR ALTERNATE SOLUTION 3.

RUN NO.	MAX. QUEUE CONTENT				QUEUE CONTENT (MOST LIKELY OCCURED)			
	BR.1	BR.2	BR.3	BR.4	BR.1	BR.2	BR.3	BR.4
1.	13	12	11	17	8	8	7	11
2.	15	13	11	17	8	8	7	11
3.	11	12	11	17	8	8	8	12
4.	19	12	10	16	9	8	7	12
5.	17	13	11	17	9	9	8	12
6.	12	12	11	22	8	8	7	13
7.	11	12	11	17	8	8	7	13
AVG.	13	12	11	18	8	8	7	12

CHAPTER SEVEN

DISCUSSION AND CONCLUSION

7.1 DISCUSSION

This thesis introduces and applies scientific engineering management techniques in evaluating any intersection, and giving best solution, by analysing the results of the output of the computer program developed to that intersection. The developed simulation program provides, a fast, easy, and reliable method of evaluating current or future conditions at the intersections. Three alternatives were simulated, the results were analyzed, and the following issues regarding modeling, data, and results are discussed:

1- In building the model, it is considered that each lane will present a facility and for each facility the queue is introduced to gather statistics for that lane. Note in branch 1, for example there are 4 facilities since there are 4 lanes. When the traffic flow enter branch 1, the flow will be distributed into the 4 lanes, but before that, there will be a check for the least queue length to enroll into it.

2- In branch 2, there is only one marked lane, but actually two cars at the same time are passing the intersection

parallel to each other, and this will introduce two queues. In order to be very accurate and present the actual situation, two lanes were considered in branch 2. This happened also in branch 4.

3- As mentioned earlier, some data was not available, so it was taken from the site. In determining the service time for branch 3', some of cars in that lane, turned left, while the others turned back (U turn), the service time for each type will differ from the other, so each type is separated by using the TRANSFER block, and certain service time was assigned to each type.

4- As known the time needed for car No.1 to cross the intersection will be greater than it needed for car No. 2, car No. 3.....until car No.5. In order to assign a fixed service time for all cars, the green time for that branch is postponed for certain time, which equals to the average time needed for the first 5 cars - average time needed for the second 5 cars in the queue.

5- In order to develop an accurate and presented program, the following was done:

- Eliminate the effect of randomness by using R-MULT card, and the same distribution function to generate arrivals.

- Run the program for complete hour, in order to reach steady state.
- The program was run seven times for each case, then the average of the seven outputs were taken as a results of that case.

6- The selected cycle times to be studied were high. Because we have 4 phases, and large traffic flow, it is recommended to increase the cycle time length, in order to allow more time for the green time period without interruption, and reduce the lost time.

7- The best solution regarding the performance and queue length in the intersection, was the third alternative, because it gave the minimum queue length, while using the forecasted traffic flow for year 2000. Alternative two will not solve the problem for long time, and it needed to buy land from it's owners, and this may be difficult. In the first alternative, it is impossible to handle such large traffic flow as in year 2000.

8- When the results of the program were compared with the existing actual situation, they were almost the same, which means that the program is present the actual situation.

9-The time needed to run the program is very small compared with other softwares, it takes about (19-28) seconds.

7.2 CONCLUSIONS

1- For the next two years the problem can be solved by adjusting the cycle time and distribute the green time properly. So the cycle time is to be 90 sec. and then increased to 120 sec., and then to 140 sec.

2- After 1993 the intersection at this situation can't handle the the forecasted traffic. So it is recommended to construct the tunnel as illustrated in the third alternate solution.

3- It is recommended to use the simulation technique approach in order to predict the results of any project before doing it.

7.3 FURTHER RESEARCH

1- Prepare all the needed data to major intersections in JORDAN, to be available when requested.

2- Study the design and construction of the tunnel in details, in order to be ready in 1993.

3- Study more than one intersection, connected together, and apply the computer program to them after modifying it.

4- Modify the program, so it can be run on (PS) computers, to make animation of the transactions.

REFERENCES

- 1- Jerry Branks , and John S. Carson ,II :Discrete - Event System Simulation " , 1984 .
- 2- Stewart V. Hoover , and Ronald Perry " Simulation , a problem solving approach " ,1989 .
- 3- Wolfgang S Homburger , and James H . Kell , "Fundamentals Of Traffic Engineering " ,1981 .
- 4- William R. Reilly , James H. Kell , and Iris J. Fullerton , " Design Of Urban Streets " , 1980 .
- 5- Theodore M. Matson , Wilbur S Smith , and Frederick W. Huro ." Traffic Engineering " 1955 .
- 6- " Transportation and Traffic Engineering Handbook " Institute Of Transportation Engineering .
- 7- R.J. Salter and M. Tadayon " Optimization Of Signal Cycle time By a Computer Simulation " , Traffic Engineering and Control 1987 ,V 28 ,part 5 , pp 290-293 .
- 8- Roger Plum and Panos Michalopoulos ," Traffic ControlAt Intersection Sites During Reconstruction " , Transportation Engineering , 1987 , V 113 , Part 4 , pp 422-436 .
- 9- James O Henriksen and Robert C. Crain " GPSS/H User Manual " ,1983 .

392822

- 10- Highway Design Manual, Ministry of Public Works , The Hashemite Kingdom of Jordan , 1972 .
- 11- Juan M. Morales and Jeffrey F. Paniati , " Two-Lane Traffic Simulation : A Field Evaluation of Roadism " , Transportation Research Record 1100, Federal Highway Administration .
- 12- David D. Bendworth and James E. Bailey "INTEGRATED PRODUCTION CONTROL SYSTEM " , second edition. Arizona state university.
- 13- William W. Hines and Douglas C. Montgomery, "PROBABILITY AND STATISTICS IN ENGINEERING AND MANAGEMENT SCIENCE. "
- 14- Thomas J. Schriber "Perspectives on simulation using GPSS", University of Michigan, Ann Arbort MI, 48109-1234, USA.
- 15- R. J. Salter "TRAFFIC ENGINEERING", University of Bradford, England.